Downtown Circulation Study

Rocky Mount, NC



Prepared for

City of Rocky Mount, NC

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Prepared by



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EXECUTIVE SUMMARY

The City of Rocky Mount has undertaken this study to assess the impact of altering traffic patterns in the downtown area. The changes to traffic patterns under consideration include converting one-way streets to two-way operation and closing a railroad crossing in the downtown area. The study area includes the area designated as the "Downtown Smart Growth Area" as identified in the Comprehensive Plan for the City of Rocky Mount. This area is defined by the intersection of Raleigh Road and Grace Street in the southwest; extending north on Grace Street to the intersection of Grace Street and West Grand Avenue; east on West Grand Avenue to Atlantic Avenue; south on Atlantic Avenue and Arlington Street to the intersection of Arlington Street and South George Street; across the railway overpass west on Raleigh Road back to the intersection of Raleigh Road and Grace Street. All of the one-way streets in the downtown area were candidates for conversion to two-way operation while the rail crossings at Hill Street/Western Avenue and Nash Street/Marigold Street were considered for closure in this study. The objective of this study was to assess the impact of the changes on traffic operations, pedestrian mobility, parking, and the economic viability of the downtown businesses. The findings of this report will provide vital information to the Rocky Mount city council on the decisions to improve the downtown area.

Six scenarios were developed for the analysis of future year conditions. The No-Build alternative includes the projected traffic volumes expected to occur in 2025, but does not include any changes to the existing street network in the downtown area. As part of developing the other five alternative analysis scenarios, it was determined that a number of existing one-way streets could be easily converted to two-way operation and included in most of the future year analysis scenarios. The following streets were considered feasible two-way conversions due to the local impacts and low cost of implementation:

- Trevathan Street
- Gay Street
- Howard Street
- Washington Street
- Rose Street
- Falls Road

In addition to the feasible one-way street conversions, the following one-way streets were analyzed as two-way streets in the alternative scenarios:

- Thomas Street
- Sunset Avenue/Tarboro Street
- Hill Street/Western Avenue
- Nash Street/Marigold Street
- One block of Southeast Main Street between Marigold Street and Hill Street

The following rail crossings were also considered for closure as part of the alternative scenarios:

- Hill Street/Western Avenue
- Nash Street/Marigold Street

Preliminary alternatives were developed and submitted for review by downtown stakeholders and by staff of the City of Rocky Mount. After incorporating feedback from stakeholders, five alternative scenarios were carried forward for analysis. The one-way

street conversions and rail crossing closures identified above were combined to formulate the following five alternative analysis scenarios:

- Alternative 1 This scenario included all of the feasible street conversions, the conversion of Main Street between Hill Street and Marigold Street to two-way operation, the conversion of the one-way pair of Hill Street/Western Avenue and Nash Street/Marigold Street to two-way operation, and the closure of the rail crossing on Nash Street/Marigold Street.
- Alternative 2 This scenario is identical to Alternative 1 with the following exceptions:
 - Washington Street and Rose Street remain one-way streets
 - Thomas Street and Sunset Avenue/Tarboro Street are converted to twoway operation.
- Alternative 3 This scenario included all of the feasible street conversions, the conversion of the one-way pair of Hill Street/Western Avenue and Nash Street/Marigold Street to two-way operation, and the closure of the rail crossing on Hill Street/Western Avenue.
- Alternative 4 This scenario is identical to Alternative 3 with the following exceptions:
 - Washington Street and Rose Street remain one-way streets
 - Thomas Street and Sunset Avenue/Tarboro Street are converted to twoway operation.
- Alternative 5 This scenario included most of the feasible one-way street conversions plus the conversion of all of the major streets in the downtown area (Thomas Street, Sunset Avenue/Tarboro Street, Hill Street/Western Avenue, Nash Street/Marigold Street, Franklin Street, and Church Street) to two-way operation. However, all railroad crossings in the downtown area remain open to vehicular crossing traffic in this scenario and Washington Street and Rose Street remain as one-way streets.

Analyses were performed to assess peak hour level of service at intersections in the study area. The level of service analyses were performed for existing conditions as well as the six scenarios of future year conditions. Existing peak hour traffic volumes were increased by an annual growth rate of two percent to account for growth of background traffic in the study area. Vehicle trips were also generated for proposed land use changes in the downtown area and were added to the background traffic to estimate peak hour traffic in year 2025.

The level of service analyses indicate that the existing signalized intersections in the study area are all operating at acceptable level of service (LOS), however the analyses also suggest that the intersections adjacent to the rail crossings in the core downtown area will be most impacted by the proposed traffic pattern changes and increased traffic volumes. The resulting LOS results are summarized in the following tables.

Alternative	Intersection Operating at Unacceptable LOS (LOS E or worse)				
Existing Conditions	None				
No-Build Conditions	None				
Alternative 1	Hill Street/Western Avenue at Main Street				
Alternative 2	Sunset Avenue/Tarboro Street at Main Street				
	Hill Street/Western Avenue at Main Street				
Alternative 3	Nash Street/Marigold Street at Main Street				
Alternative 4	Sunset Avenue/Tarboro Street at Main Street				
	Nash Street/Marigold Street at Main Street				
Alternative 5	Sunset Avenue at Church Street				
	Sunset Avenue/Tarboro Street at Main Street				

The following tables summarizes the impact of each alternative and ranks the alternatives relative to one another based on average vehicle delay and average vehicle speed.

	Ave	rage Vehicle	_			
Alternative	A.M. Peak	Rank	P.M. Peak	Rank	Average Rank	Overall Rank
No-Build Conditions	00:11	-	00:12	-	-	-
Alternative 1	00:17	1	00:19	2	1.5	2
Alternative 2	00:24	5	00:26	3	4	3
Alternative 3	00:17	1	00:14	1	1	1
Alternative 4	00:20	4	00:30	4	4	3
Alternative 5	00:19	3	00:42	5	4	3

Note: Alternatives are ranked 1 through 5, lowest average delay to highest average delay.

	Av	erage Vehic		Average		
Alternative	A.M. Peak Rank		P.M. Peak	.M. Peak Rank		Overall Rank
No-Build Conditions	14	-	13	-	-	-
Alternative 1	11	1	10	2	1.5	2
Alternative 2	9	5	9	3	4	3
Alternative 3	11	1	12	1	1	1
Alternative 4	10	4	8	4	4	3
Alternative 5	11	1	6	5	3	3

Note: Alternatives are ranked 1 through 5, highest average speed to lowest average speed.

In addition to peak hour level of service analyses, future year P.M. peak hour travel times were estimated for a set of origins and destinations for comparison between the alternative scenarios. Travel times for emergency response from Fire Station 1 were also estimated for each of the future year alternatives. As reflected in the travel time estimates, the reduction in vehicular capacity crossing the railroad tracks results in increased travel times across the study area. The table below shows how the

alternatives rank relative to one another based on the estimated P.M. peak hour travel times for the origins and destinations shown.

Alternative Ranking by P.M. Peak Hour Travel Time

		No- Build	Alt	:. 1	Alt	:. 2	Alt	. 3	Alt	. 4	Alt	. 5
Origin	Destination	Travel Time	Travel Time	Rank	Travel Time	Rank	Travel Time	Rank	Travel Time	Rank	Travel Time	Rank
Sunset	Nash Street at Main Street	3:02	2:33	1	2:53	2	3:03	3	3:22	4	3:53	5
Avenue at Grace	Hill Street at Main Street	3:47	1:59	1	4:22	3	3:42	2	5:46	5	5:31	4
Street	Marigold Street at Main Street	3:06	2:25	1	4:48	3	3:13	2	5:03	5	6:36	4
Thomas	Nash Street at Main Street	3:05	4:16	2	5:26	3	3:07	1	6:28	4	6:31	5
Street at Atlantic	Hill Street at Main Street	3:51	3:42	1	5:11	3	3:46	2	7:21	5	5:38	4
Avenue	Marigold Street at Main Street	3:10	4:08	2	5:36	3	3:17	1	6:37	5	5:45	4
Average Rank		-	1.	.6	2.8		1.8		4.6		4.3	
Overall Rank		-	1	1	67	3	2	2	Ę	5	4	1

Note: Alternatives are ranked 1 through 5, least travel time to most travel time.

Emergency vehicles are affected by changes to the downtown street operations just as other vehicles traveling in the downtown area. Analysis of estimated peak hour travel times for emergency vehicles from Fire Station 1 to the intersection of Thomas Street and Church Street indicate that Alternatives 3 and 4 would have the most significant impact on travel time. The table below ranks the alternatives relative to one another based on the estimated P.M. peak hour travel times between Fire Station 1 and the intersection of Thomas Street and Church Street. While the results indicate an increase in travel time, responding to the intersection of Thomas Street and Church Street constitutes the worst-case scenario in terms of travel time and distance from Fire Station 1. It also must be recognized that the travel times reported are for the P.M. peak hour. Travel times during off-peak periods would not be expected to reach these levels.

Alternative Ranking by Fire Station 1 P.M. Peak Hour Travel Time

Origin	Destination	No- Build	Alt. 1	Alt. 2	Alt. 3	Alt. 4	Alt. 5
Fire Station 1	Thomas Street at Church Street	2:42	4:28	4:11	5:11	6:07	3:48
Rank		-	3	2	4	5	1

Note: Alternatives are ranked 1 through 5, least travel time to most travel time.

Simtraffic analysis outputs indicate that the conversion of the east-west streets to twoway operation will result in significant increases in traffic queues on the approaches to Main Street. The table below shows the how the alternatives rank relative to one another based on estimated P.M. peak hour traffic queues.

Alternative Ranking by P.M. Peak Hour Traffic Queues at Main Street

Street	No-E	Build	Altern	ative 1	ive 1 Alternative 2		Altern	ative 3	Altern	ative 4	Alternative 5	
Street	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
Thomas St	-	115'	-	75'	25'	200'	-	115'	160'	235'	100'	120'
Sunset Ave/Tarboro St	180'	-	55'	-	340'	210'	100'	-	385'	205'	345'	235'
Western Ave/Hill St	-	145'	385'	240'	220'	280'	-	-	-	-	100'	245'
Nash St/Marigold St	55'	-	-	-	-	-	305'	160'	285'	175'	75'	75'
Average Queue	118'	130'	220'	158'	195'	230'	203'	138'	277'	205'	155'	169'
Rank by Direction	-	-	4	2	2	5	3	1	5	4	1	3
Average Rank		•	3	3	3.	.5	2	1	4	.5	2	2
Overall Rank	•	•	2	2	3	3	4	1	Ę	5	,	1

Notes: [1] The average block lengths along the east-west streets are 400' for the eastbound streets and 270' for the westbound streets.

[2] Alternatives are ranked 1 through 5, shortest queues to longest queues

Some general conclusions can also be made concerning traffic circulation and access. Either of the rail crossings under consideration for closure will significantly impact traffic circulation in the downtown area. Closing either of the railroad crossings under consideration would also require the conversion of Hill Street/Western Avenue and Nash Street/Marigold Street to two-way operation. In addition, it may be desirable to convert a block of Main Street to two-way operation to maintain an acceptable level of accessibility should the Nash Street/Marigold Street crossing be closed.

Closing the railroad crossing on Hill Street/Western Avenue would have less impact on the counterclockwise traffic circulation in the downtown than will closing the railroad crossing on Nash Street/Marigold Street. If the railroad crossing on Hill Street/Western Avenue were closed, then the existing counterclockwise circulation pattern would remain intact. However, if the Nash Street/Marigold Street rail crossing were closed, the existing counterclockwise circulation pattern would be interrupted and shortened by a block, significantly impacting the accessibility to the land uses on the block of Main Street between Hill Street/Western Avenue and Nash Street/Marigold Street.

Analyses were also performed to assess the relative economic impact of each alternative. The analyses included interviewing the downtown stakeholders to assess the current business climate in the downtown area and to gauge the economic future of the downtown area. A model was developed to assess the relative economic impacts of

each alternative based on the prevailing traffic patterns created by each alternative. The conclusions of the economic analysis is summarized below:

- Other factors besides traffic will likely influence retail sales more than traffic modifications
- By diverting additional traffic onto Main Street, Alternative 3 and Alternative 4 could potentially generate higher design year retail sales than the No-Build scenario
- Alternatives 3 and 4 create a positive impact and are projected to exceed No-Build retail sales by \$7.8 million and \$8.8 million or 14.1% and 15.9% respectively (average annual change of 0.6% and 0.7% respectively)
- Alternatives 1, 2, and 5 are projected to generate fewer design year retail traffic and sales than the No-Build scenario decreasing sales by \$3.0 million, \$2.4 million, and \$3.6 million or 5.5%, 4.5%, and 6.5% respectively (average annual changes of 0.25%, 0.20%, and 0.29% respectively)
- All alternatives generate slightly more retail traffic than No-Build in the general area between the intersections of Main Street with Sunset Avenue and Hill Street/Western Avenue with Alternatives 3 and 4 generating moderately more retail traffic

The table below summarizes the impact of each alternative on retail sales in the downtown area.

	No-Build	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Annual Retail Sales	\$55.5 M	\$52.5 M	\$53.0 M	\$63.40 M	\$64.37 M	\$51.0
Rank	-	4	3	2	1	5

Implementation costs for the improvements required for each alternative were developed. The preliminary cost estimates include items ranging from the cost of upgrading pavement markings in order to accommodate two-way traffic to realigning a portion of Sunset Avenue and Thomas Street in the vicinity of City Lake to facilitate the conversion of Sunset Avenue and Thomas Street to two-way operation. Cost estimates across the five (5) build alternatives range from \$600,000 to \$2,300,000.

Alternative Ranking by Implementation Cost

Alternative	Anticipated Implementation Cost	Rank
Alternative 1	\$670,000	2
Alternative 2	\$1,900,000	4
Alternative 3	\$600,000	1
Alternative 4	\$1,800,000	3
Alternative 5	\$2,300,000	5

Findings

A number of measures were developed in this study to assess the impact of each alternative on traffic operations in the downtown area. The alternatives have been ranked relative to one another based on each of these measures. The table below summarizes those rankings based on average vehicle delay, average vehicle speed, average P.M. peak hour traffic queues at Main Street, average P.M. peak hour travel time between specified origins, average P.M. peak hour travel times between Fire Station 1 and the intersection of Thomas Street and Church Street, anticipated Implementation Costs, and impact on retail traffic.

Overall Alternative Ranking

Alternative	Average Vehicle Delay	Average Vehicle Speed	Average P.M. Peak Hour Queues	Average P.M. Peak Hour Travel Times	Average Fire Station 1 P.M. Peak Hour Travel Times	Anticipated Implementation Cost	Impact on Retail Traffic	Average Ranking	Overall Ranking
1	2	2	2	1	3	2	4	2.3	2
2	3	3	3	3	2	4	3	3.0	3
3	1	1	4	2	4	1	2	2.1	1
4	3	3	5	5	5	3	1	3.6	5
5	3	3	1	4	1	5	5	3.1	4

In comparison to the No-Build scenario, all of the future year alternatives result in increased delays and travel times. The level of service analyses indicate that the intersections adjacent to the railroad tracks on Main Street are the most impacted by the proposed traffic pattern changes. Converting streets from one-way operation reduces the capacity to move vehicles through the downtown area, increases travel delays, and could possibly result in traffic diverting around downtown. In addition, closing one of the four railroad crossings in the downtown area significantly disrupts existing traffic patterns in the area, impairs mobility, reduces accessibility, increases delays, and raises the likelihood that motorists will divert around downtown during peak travel periods.

Estimates of P.M. peak hour travel time indicate that as more of the east-west streets are converted to two-way operation, travel time through the study area will increase. This is due to the loss of capacity at the intersections adjacent on Main Street and the loss of a travel lane in each direction for carrying vehicles.

In terms of economic impact, analyses indicate that Alternative 3 and Alternative 4 would generate more retail traffic than the No-Build alternative while Alternative 1, Alternative 2, and Alternative 5 would generate slightly less retail traffic than the No-Build Alternative.

Recommendations

This report includes a relatively detailed evaluation of the implications of closing one of the existing at-grade railroad crossings and the conversion of one-way streets to two-way operation in the downtown area. However, in addition to the impacts measured in this report, there are a wide range of other impacts that must factor into any decision concerning changes to traffic circulation in the downtown study area. With that said, the

intent of this study is to quantify the impacts of the various alternatives under consideration in terms of traffic operations and economic impact and to compare the alternatives to one another based on those impacts.

The alternatives considered in this study result have varying impacts on traffic operations and the economic outlook in the downtown area. However, based on the results of the analyses in this study, Alternative 3 ranks the highest in comparison to all the other alternatives. If implemented, Alternative 3 would be expected to result in increased delay in the study area, but it is expected to have a positive impact on retail traffic in the downtown area and to have the lowest implementation cost of all the alternatives considered.

The improvements required to implement Alternative 3 are estimated to cost \$600,000. Alternative 3 requires the following improvements:

- Remove the traffic signal and railroad crossing devices at the intersection of Hill Street/Western Avenue,
- Upgrade 12 signalized intersections along Nash Street/Marigold Street and Hill Street/Western Avenue between George Street and Grace Street to accommodate two-way traffic,
- Upgrade the railroad protection devices (gates and flashers) at Nash Street/Marigold Street to accommodate two-way traffic,
- Remove the traffic signal at Hill Street and Washington Street,
- Reconstruct the intersection of George Street and Marigold Street to increase the turning radius for fire trucks turning right from George Street onto Marigold Street.
- Install pavement markings along Nash Street/Marigold Street and Hill Street/Western Avenue and upgrade pavement markings at signalized intersections from George Street to Grace Street in order to accommodate twoway traffic.

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1.0 INTRODUCTION

The City of Rocky Mount has undertaken this study to assess the impact of altering traffic patterns in the downtown area. The changes to traffic patterns under consideration include converting one-way streets to two-way operation and closing a railroad crossing in the core downtown area. The study area includes the area designated as the "Downtown Smart Growth Area" as identified in the Comprehensive Plan for the City of Rocky Mount. This area is defined by the intersection of Raleigh Road and Grace Street in the southwest; extending north on Grace Street to the intersection of Grace Street and West Grand Avenue; east on West Grand Avenue to Atlantic Avenue; south on Atlantic Avenue and Arlington Street to the intersection of Arlington Street and South George Street: across the railway overpass west on Raleigh Road back to the intersection of Raleigh Road and Grace Street. All of the one-way streets in the downtown area were candidates for conversion to two-way operation while the rail crossings at Hill Street/Western Avenue and Nash Street/Marigold Street were considered for closure in this study. The objective of this study was to assess the impact of the changes on traffic operations, pedestrian mobility, parking, and the economic viability of the downtown businesses for five alternative scenarios. The horizon year is 2025.

Preliminary alternatives were developed and submitted for review. After incorporating feedback from stakeholders in the downtown area, five alternative scenarios were carried forward for analysis. The one-way street conversions and rail crossing closures were combined to formulate the following five alternative analysis scenarios:

- Alternative 1 This scenario included all of the feasible street conversions, the conversion of Main Street between Hill Street and Marigold Street to two-way operation, the conversion of the one-way pair of Hill Street/Western Avenue and Nash Street/Marigold Street to two-way operation, and the closure of the rail crossing on Nash Street/Marigold Street.
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2.0 EXISTING CONDITIONS

This section describes the existing streets and land uses in the study area. The study area includes the area designated as the "Downtown Smart Growth Area" as identified in the Comprehensive Plan for the City of Rocky Mount. This area, shown in Figure 1, is defined by the intersection of Raleigh Road and Grace Street in the southwest; extending north on Grace Street to the intersection of Grace Street and West Grand Avenue; east on West Grand Avenue to Atlantic Avenue; south on Atlantic Avenue and Arlington Street to the intersection of Arlington Street and South George Street; across the railway overpass west on Raleigh Road back to the intersection of Raleigh Road and Grace Street. The majority of the land uses in the study area are retail, commercial, and government.

STREETS AND TRAFFIC

Average Annual Daily Traffic (AADT) data for the roads in the study area were obtained from the North Carolina Department of Transportation (NCDOT). The most recent AADT counts from the NCDOT are for 2001. Figure 2 and Figure 3 provide schematic diagrams of the roadways in the study area including the existing intersection geometrics and peak hour turning movement volumes.

Thomas Street and Sunset Avenue/Tarboro Street

 These streets form a one-way pair in the northern half of the study area running in the east-west direction and are designated as Business Route US 64. Both streets have three-lane cross-sections through the study area and have a posted speed limit of 25 miles per hour. The 2001 AADT on Thomas Street was between Church Street and Southwest Main Street was 4,700 vehicles per day (VPD). The 2000 AADT on Sunset Avenue at the CSX railroad crossing was 5,100 VPD.

Hill Street/Western Avenue and Nash Street/Marigold Street

 These streets form a one-way pair in the southern half of the study area running in the east-west direction. Both streets have two-lane to three-lane cross-sections through the study area and have a posted speed limit of 25 miles per hour (MPH). These streets provide access to residential areas east and west of the downtown area.

Main Street

 Main Street operates as a one-way pair of streets in the downtown area separated by railroad tracks and provides local access to the shops in the downtown area. Two travel lanes are provided in each direction with angled, on-street parking. Main Street intersects the east-west streets forming very closely spaced intersections at the railroad tracks.

Franklin Street and Church Street

• These streets form a one-way pair in the western section of the study area running in the north-south direction. Both streets have three-lane cross-sections through the study area and have a posted speed limit of 25 MPH and are designated as Business Route US 301.

Washington Street and Rose Street

 These streets form a local one-way pair between Tarboro Street and Thomas Street with a posted speed limit of 25 MPH. On-street parking is provided on these streets. South of Sunset Avenue, Washington Street is a two-way street with on-street parking provided.

Arlington Street/Atlantic Avenue

 This street marks the eastern boundary of the study area. The street has a four-lane cross-section with a posted speed limit of 25 and 35 MPH.

Trevathan Street

• This street is actually outside of the defined study area. The street is a one-way street with on-street parking provided.

Gay Street

 This street is a one-way street in the northwestern section of the study area providing local access to commercial land uses.

Howard Street

• This street is a north-south one-way street providing one-way access between Western Avenue and Sunset Avenue west of Southwest Main Street. The land uses along this street are primarily commercial and retail.

Falls Road

 This street is a north-south one-way street providing one-way access between Goldleaf Street and Thomas Street in the northern section of the study area. This street provides access from areas north of the downtown area.

Peak hour intersection volumes were provided by the City of Rocky Mount. The data, which was collected during 2003, provided A.M. and P.M. peak hour intersection traffic volumes in fifteen-minute increments. The traffic volumes were utilized to assess existing peak hour level of service and served as the basis for estimating future year turning movements. The peak hour traffic volume data, as well as the AADT data for the study area, is provided in Appendix B.

The existing intersection turning movement data indicates that the majority of the traffic approaching the railroad crossings from the east and west is traveling through downtown without diverting onto Southwest Main Street or Southeast Main Street. The existing A.M. peak hour and P.M. peak hour traffic volumes are shown in Figure 2 and Figure 3.

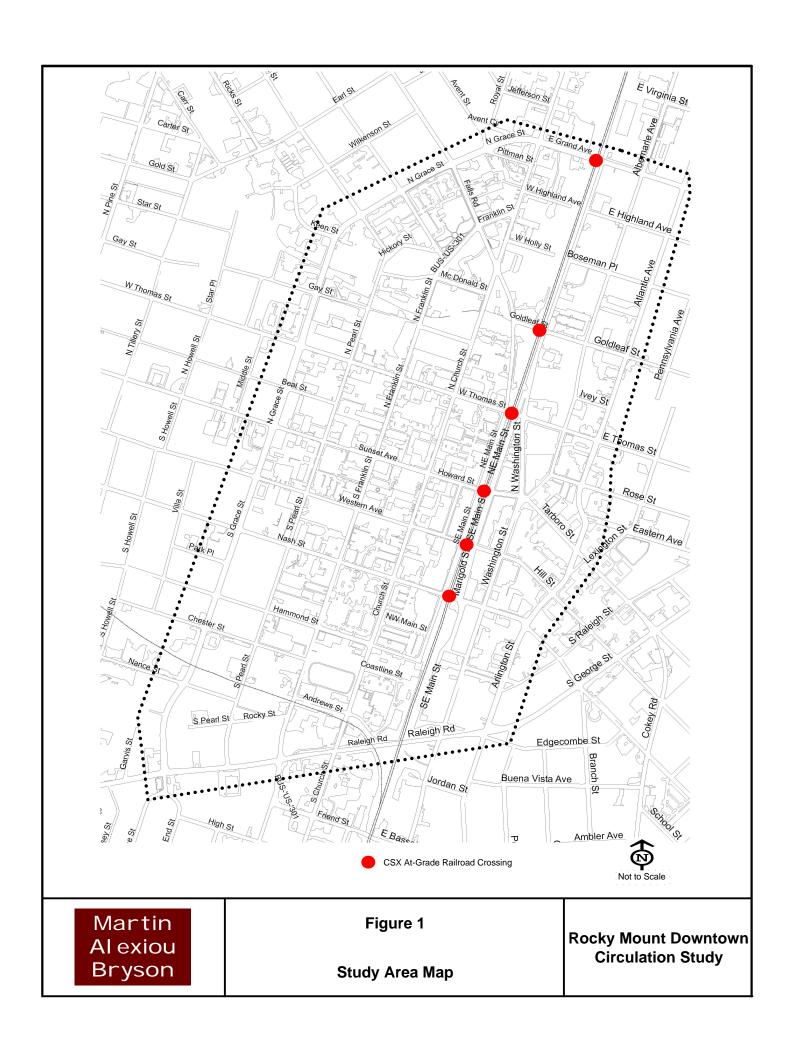
CSX RAILROAD

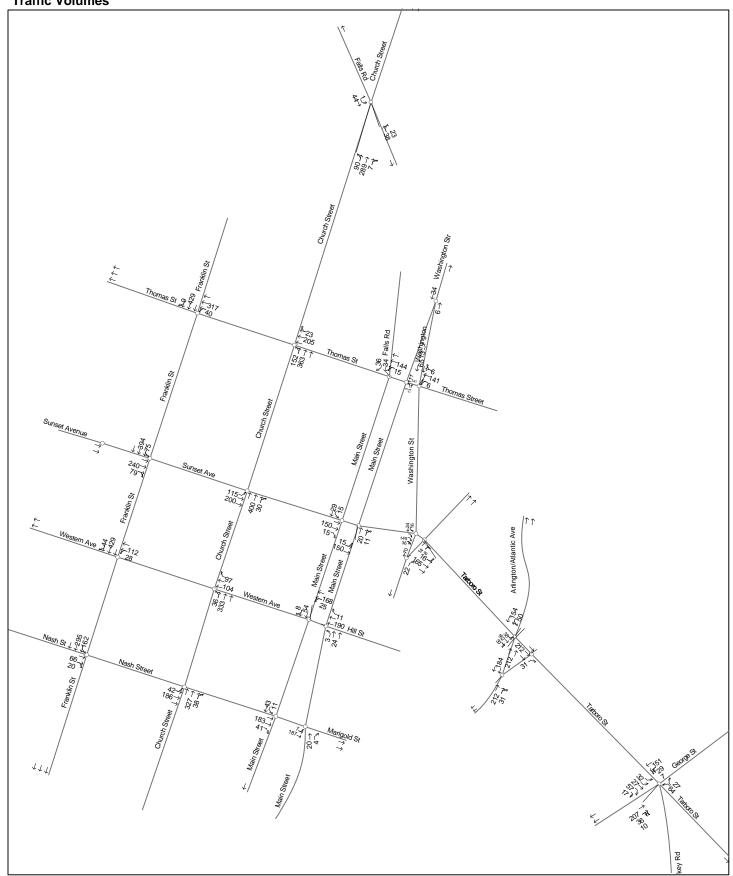
A CSX railroad line is oriented north-south passing through the downtown area. The at-grade crossings are controlled by traffic signals, gates, and flashers. According to signal pre-emption data, the average duration of the closures of the railroad crossings vary from an average minimum of approximately 1:37 to an average maximum of approximately 2:36. During the period of March 1, 2005, through March 7, 2005, the railroad crossings at Main Street were closed for trains to pass through nearly 300 times, an average of



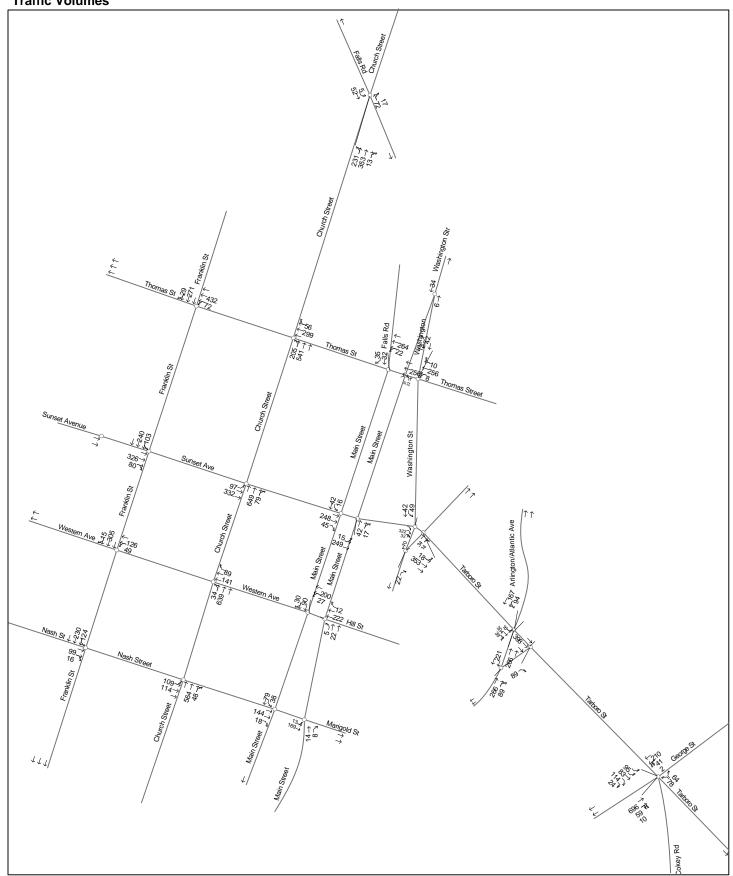
Over 40 trains pass through downtown each day.

approximately 40 times each day. The overall average duration of the closures was 2:09. The railroad crossings are indicated in Figure 1 and the rail crossing data is provided in Appendix A.





Rocky Mount Downtown Circulation Study



Rocky Mount Downtown Circulation Study

3.0 LAND USE, FUTURE YEAR (2025) TRIP GENERATION, AND DISTRIBUTION

LAND USE

The proposed land use is a vital component of the analyses of future year traffic conditions as well as the analysis of the economic impact in the downtown area. The City provided projected land use data for each block in the downtown area. The following land use types are proposed in the downtown area:

Residential

Public

Industrial

Office

Transportation

Parks

Commercial

Multi-use

Only the blocks indicated in Figure 3 were included for trip generation in this study. The following assumptions were made concerning residential, industrial, and multi-use land uses.

- Two-thirds of the square footage designated as multi-use was assumed to be retail, while the remaining one-third square footage was assumed to be office space.
- o Residential units were assumed to be 1,000 square feet in size.
- Commercial development was designated as specialty retail for the purposes of trip generation.
- An occupancy rate of 85% was assumed for the residential, commercial, and office space.

Table 1 Detailed Land Use By Block

Block Number	Residential Units ¹	Commercial ¹ (square feet)	Public (square feet)	Office ¹ (square feet)
101	27	4,900	55,642	6,877
103	33	15,259	54,192	30,518
104	16	10,042	0	20,084
105	33	22,290	0	29,398
106	33	10,892	20,271	21,784
107	38	21,069	2,264	32,100
108	55	21,817	18,634	43,633
109	26	10,459	0	20,917
111	0	14,292	0	28,585
110	0	0	0	0
112	0	43,406	0	7,235
113	0	2,742	0	5,483
114, 115, & 116 ²	59	36,359	16,700	0
313	0	0	144,552	0
Totals	320	213,527	312,255	246,614

^{1 -} Assumes 85% occupancy.

^{2 -} Douglas Block.

TRIP GENERATION

A.M. peak hour and P.M. peak hour vehicle trips were generated only for blocks within the core downtown area. It was assumed that trips generated by the proposed downtown businesses and other land uses would be in addition to the existing business related trips. Table 3 includes a summary of the trip generation and detailed land use data for each block is included in Appendix C. Most of the trip generation rates utilized in this study were derived from the ITE Trip Generation Manual 7th Edition.

In cases where applicable trip generation rates are not included in the ITE Trip Generation Manual, data were obtained from similar facilities in other jurisdictions to estimate trip generation. For example, the City of Rocky Mount proposes to build an

expanded Children's Museum in the downtown area. Trip rates for children's museums are not included in the ITE Trip Generation Manual; therefore data obtained from websites and staff members of similar facilities were used to estimate daily trips. In order to estimate peak hour trips, trip generation rate information from the ITE Trip Generation Manual for libraries was used to estimate the ratio of peak hour trips to daily trips. That ratio was then utilized to estimate the peak hour trips for children's museums. Table 3 summarizes the trip generation for the blocks of interest in the downtown area.

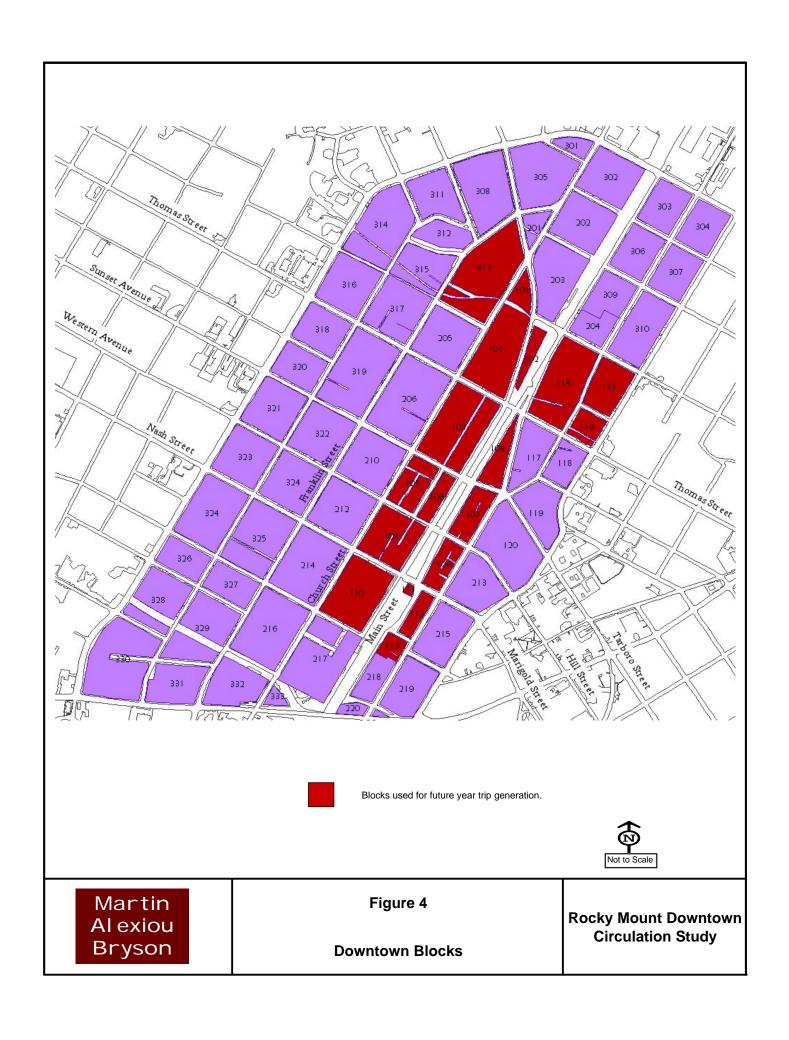


Buildings on Main Street are slated for renovations.

Table 2 Trip Generation Summary by Block

Block Number	AM Peak Hour Trips	PM Peak Hour Trips
101	97	78
103	207	143
104	109	66
105	213	124
106	132	131
107	215	134
108	253	202
109	118	75
111	142	81
110	0	0
112	308	129
113	27	15
114, 115, & 116	278	135
313	49	326
Totals	2148	1639

Details of the land use and related trip generation are included in Appendix C.



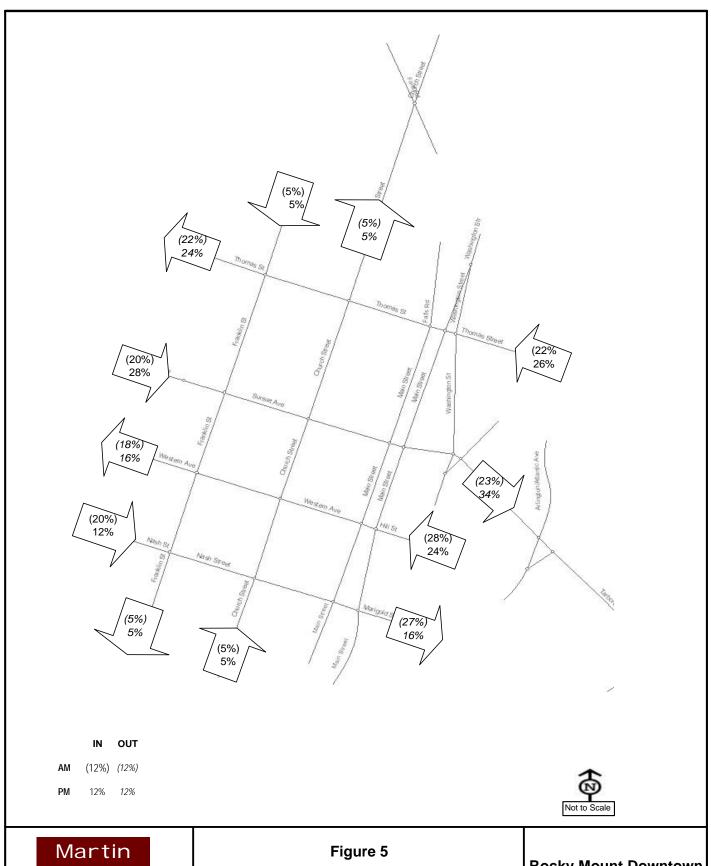
TRIP DISTRIBUTION

In order to assess the economic impact of each alternative, phone surveys were conducted with the business owners in the downtown area. Based on those survey results, it was estimated that 50 percent of the existing downtown business trips are generated in Nash County and 50 percent is generated in Edgecombe County. Therefore, as a beginning point in distributing the traffic generated by the proposed land uses, it was assumed that 50 percent of the traffic would be attributed to Nash County and 50 percent of the traffic would be attributed to Edgecombe County. The trips were then distributed across the streets in the downtown area. The overall trip distribution is summarized in Table 3 and shown in Figure 5.

Table 3 Trip Distribution Percentages

Approach	A.M. Peak Hour		P.M. Peak Hour	
Арргодоп	In	Out	In	Out
From the North on Franklin Street	5%	-	5%	-
From the South on Church Street	5%	-	5%	-
From the West on Sunset Avenue	20%	-	28%	-
From the West on Nash Street	20%	-	12%	-
From the East on Thomas Street	22%	-	26%	-
From the East on Hill Street	28%	-	24%	-
To the North on Church Street	ı	5%	ı	5%
To the South on Franklin Street	-	5%	-	5%
To the West on Thomas Street	-	22%	-	24%
To the West on Western Avenue	-	18%	-	16%
To the East on Tarboro Street	-	23%	-	34%
To the East on Marigold Street	=	27%	=	16%

The trips generated by the land uses within the blocks identified in Table 2 were distributed through the network based on the percentages identified in Table 3 and the existing street operations. The resulting intersection turning movement volumes were used to establish the peak hour turning movement volumes for the No-Build alternative. The No-Build alternative turning movement volumes were then redistributed for each alternative based on changes to the streets included in each alternative.



Martin Al exiou Bryson

A.M. Peak Hour and P.M. Peak Hour Trip Distribution

Rocky Mount Downtown Circulation Study

4.0 ALTERNATIVE ANALYSIS SCENARIOS

Five alternative analysis scenarios, plus a No-Build scenario, were developed to analyze proposed future year conditions. The Future No-Build analysis scenario includes the existing traffic as described in Section 2.0 of this report, plus 2.0% growth between the existing year for which base traffic data were provided (2003) and the build years (2025), as well as traffic from the proposed land uses in the downtown area, as discussed in Section 3.0 of this report. The following sections describe the alternative scenarios.

SCENARIO DESCRIPTIONS

In all, six scenarios were developed to evaluate future year conditions. The No-Build alternative includes the projected traffic volumes expected to occur in 2025, but does not consider any changes to the existing street network in the downtown area. As part of developing the other five alternative analysis scenarios, it was determined that a number of existing one-way streets could be easily converted to two-way operation and included in most of the future year analysis scenarios. The following streets were considered feasible two-way conversions due to the relatively insignificant impacts and low cost of implementation:

- Trevathan Street
- Gay Street
- Howard Street
- Washington Street
- Rose Street
- Falls Road

In addition to the feasible one-way street conversions, the following one-way streets were analyzed as two-way streets in the alternative scenarios:

- Thomas Street
- Sunset Avenue/Tarboro Street
- Hill Street/Western Avenue
- Nash Street/Marigold Street
- One block of Southeast Main Street between Marigold Street and Hill Street

The following rail crossings were also considered for closure as part of the alternative scenarios:

- Hill Street/Western Avenue
- Nash Street/Marigold Street

Preliminary alternatives were developed and submitted for review. After incorporating feedback from stakeholders in the downtown area, five alternative scenarios were carried forward for analysis. The one-way street conversions and rail crossing closures identified above were combined to formulate the following five alternative analysis scenarios:

 No-Build – This alternative included the estimated peak hour traffic volumes for year 2025, including new trips for the proposed land uses in the downtown core, but does not include any changes to the existing street network.

- Alternative 1 This alternative included all of the feasible street conversions, the
 conversion of Main Street between Hill Street and Marigold Street to two-way
 operation, the conversion of the one-way pair of Hill Street/Western Avenue and
 Nash Street/Marigold Street to two-way operation, and the closure of the rail
 crossing on Nash Street/Marigold Street.
- Alternative 2 This alternative is identical to Alternative 1 with the following exceptions:
 - Washington Street and Rose Street remain one-way streets
 - Thomas Street and Sunset Avenue/Tarboro Street are converted to twoway operation.
- Alternative 3 This alternative included all of the feasible street conversions, the conversion of the one-way pair of Hill Street/Western Avenue and Nash Street/Marigold Street to two-way operation, and the closure of the rail crossing on Hill Street/Western Avenue.
- Alternative 4 This alternative is identical to Alternative 3 with the following exceptions:
 - Washington Street and Rose Street remain one-way streets
 - Thomas Street and Sunset Avenue/Tarboro Street are converted to twoway operation.
- Alternative 5 This alternative included most of the feasible one-way street conversions plus the conversion of all of the major streets in the downtown area (Thomas Street, Sunset Avenue/Tarboro Street, Hill Street/Western Avenue, Nash Street/Marigold Street, Franklin Street, and Church Street) to two-way operation. However, all railroad crossings in the downtown area remain open to vehicular crossing traffic in this scenario and Washington Street and Rose Street remain as one-way streets.

ESTIMATED PEAK HOUR TRAFFIC VOLUMES

In order to assess the impacts of the alternatives, it was necessary to redistribute the future year traffic volumes included in the No-Build scenario taking into account the changes to the street network in each alternative. The assumptions that were made in order to develop the peak hour traffic volumes for each alternative are discussed below.

- No-Build Peak hour traffic volumes for this alternative were estimated based on growth of existing traffic at a rate of two percent per year and the added trips generated by the proposed land use changes in the downtown core. The trips generated by the new land uses were distributed throughout the downtown street network by the percentages indicated in Table 3. The A.M. peak hour and P.M. peak hour traffic volumes for this alternative are shown in Figures 8 and 9.
- Alternative 1 It was assumed that 90% of the traffic traveling on Hill Street/Western Avenue and Nash Street/Marigold Street in the No-Build alternative would travel on Hill Street/Western Avenue and 10% would travel on Nash Street/Marigold Street. As part of this alternative, the rail crossing on Nash Street/Marigold Street would be closed; therefore all of the traffic crossing the railroad tracks in the No-Build alternative on this street was rerouted to cross the railroad tracks on Hill Street/Western Avenue. Traffic patterns were also adjusted to reflect the proposed conversion of Main Street to two-way operation.

Some of the traffic patterns on Thomas Street and Sunset Avenue were also adjusted to reflect the proposed conversion of Washington Street, Rose Street, and Falls Road to two-way operation. The A.M. peak hour and P.M. peak hour traffic volumes for this alternative are shown in Figures 10 and 11.

 Alternative 2 – The assumptions used to estimate traffic on Hill Street/Western Avenue and Nash Street/Marigold Street in this alternative were the same as in Alternative 1.

It was assumed that 70% of the traffic traveling on Sunset Avenue and Thomas Street would travel on Sunset Avenue and the remaining 30% would travel on Thomas Street. It was further assumed that Washington Street and Rose Street would remain under one-way operation for this alternative. If Washington Street and Rose Street were converted to two-way operation for this alternative, the resulting traffic operations at the intersection of Tarboro Street at Washington Street and Rose Street (Five Points) and at the intersection of Main Street and Thomas Street would become more complex thus resulting in severely congested peak hour traffic conditions. The A.M. peak hour and P.M. peak hour traffic volumes for this alternative are shown in Figure 12 and 13.

• Alternative 3 – It was assumed that 10% of the traffic traveling on Hill Street/Western Avenue and Nash Street/Marigold Street in the No-Build alternative would travel on Hill Street/Western Avenue and 90% would travel on Nash Street/Marigold Street. As part of this alternative, the rail crossing on Hill Street/Western Avenue would be closed; therefore all of the traffic crossing the railroad tracks on this street in the No-Build alternative was rerouted to cross the railroad tracks on Nash Street/Marigold Street. It was also assumed that no blocks of Main Street or Main Street would be converted to two-way operation.

Some of the traffic patterns on Thomas Street and Sunset Avenue were adjusted to reflect the proposed conversion of Washington Street, Rose Street, and Falls Road to two-way operation. The A.M. peak hour and P.M. peak hour traffic volumes for this alternative are shown in Figure 14 and 15.

 Alternative 4 – The assumptions used to estimate traffic on Hill Street/Western Avenue and Nash Street/Marigold Street in this alternative were the same as in Alternative 3.

The assumptions used to estimate traffic on Thomas Street and Sunset Avenue for this alternative are the same was the assumptions used to estimate traffic on these streets in Alternative 2. The A.M. peak hour and P.M. peak hour traffic volumes for this alternative are shown in Figure 16 and 17.

 Alternative 5 – It was assumed that 70% of the traffic traveling on Hill Street/Western Avenue and Nash Street/Marigold Street in the No-Build alternative would travel on Hill Street/Western Avenue and 30% would travel on Nash Street/Marigold Street.

It was assumed that 70% of the traffic traveling on Sunset Avenue and Thomas Street would travel on Sunset Avenue and the remaining 30% would travel on Thomas Street.

It was assumed that 70% of the traffic traveling on Church Street and Franklin Street would travel on Church Street and the remaining 30% would travel on Franklin Street. The A.M. peak hour and P.M. peak hour traffic volumes for this alternative are shown in Figure 18 and 19.

IMPACTS ON PARKING

Availability of convenient parking is a vital component to economic development in the downtown area. On-street parking is currently provided in the downtown area in the form of angled parking on both sides of Main Street. Stakeholders expressed a strong desire to maintain the current level of on-street parking in the downtown area. Stakeholders also wanted to investigate opportunities for adding more parking downtown. However, it is anticipated that converting blocks of Main Street to two-way operation would result in a moderate loss of on-street parking, if the parking were designed to current city

guidelines. According to the city of Rocky Mount design guidelines the minimum width necessary to provide 60-degree on-street parking on one side of a two-way street and parallel parking on the other side of the street is approximately 53.9 feet. Main Street is approximately 49 feet wide between Hill Street and Marigold Street. If this block of Main Street were converted to two-way traffic and the minimum dimensions were utilized for 60-degree angled parking, the parallel parking on Main Street would be removed.



Angled parking is provided on Main Street.

5.0 TRAFFIC ANALYSES

LEVEL OF SERVICE CRITERIA

Peak hour level of service (LOS) measures the adequacy of the intersection geometrics and traffic controls of a particular intersection or approach for the given turning movement volumes. Levels of service range from A through F, based on the average control delay experienced by vehicles traveling through the intersection during the peak hour. Control delay represents the portion of total delay attributed to traffic control devices (e.g., signals or stop signs). The engineering profession generally accepts LOS D as an acceptable operating condition for signalized intersections in urban areas and LOS C for rural areas.

At unsignalized intersections, a LOS E is generally considered acceptable only if the side street encounters delay. Nevertheless, side streets typically function at LOS F during peak traffic periods, because the traffic volumes often do not warrant a traffic signal to assist side street traffic. Table 5 provides a general description of the various levels of service categories and delay ranges.

Table 4 Intersection Level of Service Descriptions Based on Average Delay

Level of Service	Description	Signalized Intersection	Unsignalized Intersection
А	Little or no delay	<= 10 sec.	<= 10 sec.
В	Short traffic delay	10-20 sec.	10-15 sec.
С	Average traffic delay	20-35 sec.	15-25 sec.
D	Long traffic delay	35-55 sec.	25-35 sec.
E	Very long traffic delay	55-80 sec.	35-50 sec.
F	Unacceptable delay	> 80 sec.	> 50 sec.

LEVEL OF SERVICE RESULTS

Intersection levels of service analyses were performed for the typical weekday A.M. and P.M. peak hours using *Synchro/SimTraffic Professional Version 6*. The analyses were performed for existing conditions, future year (2025) no-build conditions, and for the future year (2025) alternatives described in Section 4 of this report. The level of service results are summarized in Table 6 and detailed *Synchro* output is available in Appendix D. The analyses were performed for each alternative with the following assumptions:

- Left-turn lanes would be provided where possible when streets are converted from one-way operation to two-way operation.
- Traffic signal phasing was assumed to be as simple as possible. For example, although left-turn conflict is introduced at the intersections on Church Street and Franklin Street as a result of converting streets to two-way operation, simple twophase operation was utilized for those intersections in all alternatives.

Table 5 Level of Service Results Summary

Alternative	Intersection Operating at Unacceptable LOS (LOS E or worse)
Existing Conditions	• None
No-Build Conditions	• None
Alternative 1	Hill Street/Western Avenue at Main Street /Main Street
Alternative 2	Sunset Avenue/Tarboro Street at Main StreetHill Street/Western Avenue at Main Street
Alternative 3	Nash Street/Marigold Street at Main Street
Alternative 4	Sunset Avenue/Tarboro Street at Main StreetNash Street/Marigold Street at Main Street
Alternative 5	Sunset Avenue at Church StreetSunset Avenue/Tarboro Street at Main Street

Peak hour traffic volumes and levels of service for each alternative are shown in Figures 6 through 19.

The level of service results indicate that the intersections adjacent to the railroad tracks (Main Street) are the most affected by the changes in traffic associated with the various alternatives. This is due to the geometry of the intersections that will result once the streets crossing the railroad are converted to two-way operation. Because of the location of the railroad crossings, the intersections adjacent to the railroad tracks are extremely wide. The location of the railroad tracks in close proximity to Main Street creates two very closely spaced intersections on each side of the railroad tracks that operate as a single intersection. Fortunately, with the east-west streets being one-way, it is possible to provide an acceptable level of service. However, should the east-west streets be converted to two-way operation, the signal phasing would become much more complex. With such a wide intersection and the close proximity of the railroad tracks, it is not feasible to allow permitted left-turn movements. Therefore, each approach to the intersection requires a protected phase within the traffic signal cycle. This increases the signal cycle length and increased delays for each approach to the intersection. This effect is apparent in the level of service results for Alternative 1 through 5. The following sections discuss the analyses results for each alternative.

Existing Conditions

The analysis results for existing conditions indicate that, with the exception of delays caused by trains passing through downtown, the downtown street network has excess capacity. Currently all of the intersections analyzed in this study are operating at an acceptable level of service.

No-Build Conditions

The analysis results for No-Build conditions indicated that, with the exception of delays expected to occur due to trains passing through downtown, the downtown street network has excess capacity. All of the intersections analyzed are expected to operate at an acceptable level of service.

Alternative 1

The analysis results for Alternative 1 indicate that the proposed changes to traffic patterns will negatively impact the intersection of Hill Street/Western Avenue with Main Street. As explained above, this is due the complexity of traffic operations required at this intersection if Hill Street/Western Avenue is converted to two-way operation.

Alternative 2

The analysis results for Alternative 2 indicate that the proposed changes to traffic operations will negatively impact the intersection of Hill Street/Western Avenue with Main Street. Again this is due the complexity of traffic operations required at this intersection if Hill Street/Western Avenue is converted to two-way operation.



The railroad tracks through downtown will contribute to the complexity of signal phasing once the east-west streets are converted to two-way operation.

The analysis results for Alternative 2 also indicate that the proposed changes to traffic operations will negatively impact the intersection of Sunset Avenue/Tarboro Street at Main Street. Again this is due the complexity of traffic operations required at this intersection if Sunset Avenue/Tarboro Street is converted to two-way operation.

Alternative 3

The analysis results for Alternative 3 indicate that the proposed changes to traffic operations will negatively impact the intersection of Nash Street/Marigold Street at Main Street. Again this is due the complexity of traffic operations required at this intersection if Nash Street/Marigold Street is converted to two-way operation. In addition, the intersection of Marigold Street and Main Street is currently unsignalized. It is assumed that if this alternative is implemented then the intersection will be signalized and operated with the same controller as the intersection of Nash Street and Main Street.

Alternative 4

The analysis results for Alternative 4 indicate that the proposed changes to traffic operations will negatively impact the intersection of Nash Street/Marigold Street at Main Street. Again this is due the complexity of traffic operations required at this intersection if Nash Street/Marigold Street is converted to two-way operation. In addition, the intersection of Marigold Street and Main Street is currently unsignalized. It is again assumed that if this alternative is implemented then the intersection will be signalized and operated with the same controller as the intersection of Nash Street and Main Street.

The analysis results for Alternative 4 also indicate that the proposed changes to traffic operations will negatively impact the intersection of Sunset Avenue/Tarboro Street at Main Street. Again this is due the complexity of traffic operations required at this intersection if Sunset Avenue/Tarboro Street is converted to two-way operation.

Alternative 5

The analysis results for Alternative 5 indicate that the proposed changes to traffic operations will negatively impact the intersection of Sunset Avenue/Tarboro Street at

Main Street. Again this is due the complexity of traffic operations required at this intersection if Sunset Avenue/Tarboro Street is converted to two-way operation.

The analysis results for Alternative 5 also indicate that the proposed changes to traffic operations will negatively impact the intersection of Sunset Avenue at Church Street. One of the assumptions of this alternative is that Sunset Avenue will carry 70% of the east-west traffic projected to travel on Sunset Avenue and Thomas Street. It was also assumed that Church Street would carry 70% of the north-south traffic projected to travel on Church Street and Franklin Street. Converting these streets to two-way operation also introduces conflicts for left-turning vehicles at this intersection. The combined effect of increased traffic and decreased intersection capacity results in an unacceptable level of service at this intersection.

The intersection of Marigold Street and Main Street is currently unsignalized. It is again assumed that if this alternative is implemented, then the intersection will be signalized and operated with the same controller as the intersection of Nash Street and Main Street.

CONCLUSIONS

In comparison to the No-Build alternative, all of the alternatives under consideration result in increased travel delays for vehicles traveling through downtown on the east-west streets, however the impact of some alternatives are greater than others. Table 6 summarizes the delay for vehicles traveling through downtown for each alternative. This measure is useful to compare the alternatives one against the other. The alternatives are ranked in order of average vehicle delay from lowest average vehicle delay to highest average vehicle delay.

Table 6 Average Vehicle Delays

	Network Vehicle Delay (sec/veh) [1]					
Alternative	A.M. Peak	Rank	P.M. Peak	Rank	Average Rank	Overall Rank
No-Build Conditions	00:11	-	00:12	-	-	-
Alternative 1	00:17	1	00:19	2	1.5	2
Alternative 2	00:24	5	00:26	3	4	3
Alternative 3	00:17	1	00:14	1	1	1
Alternative 4	00:20	4	00:30	4	4	3
Alternative 5	00:19	3	00:42	5	4	3

Note: Alternatives are ranked 1 through 5, lowest average delay to highest average delay.

[1] Summarizes average delay for all vehicles in the study area.

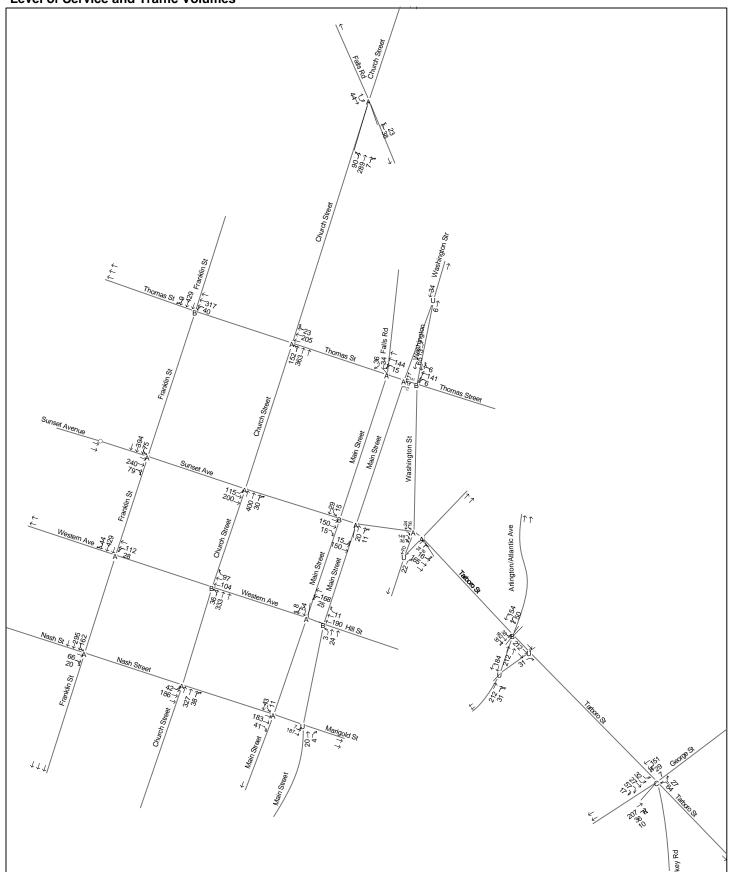
In addition to comparing network system delay across the alternative, network average speed is also a useful measure to compare the alternatives. Table 7 summarizes the average vehicle travel speeds on the roads in the study area for each alternative. The alternatives are also ranked in order of average travel speed from highest average speed to lowest average speed.

Table 7 Average Vehicle Speeds

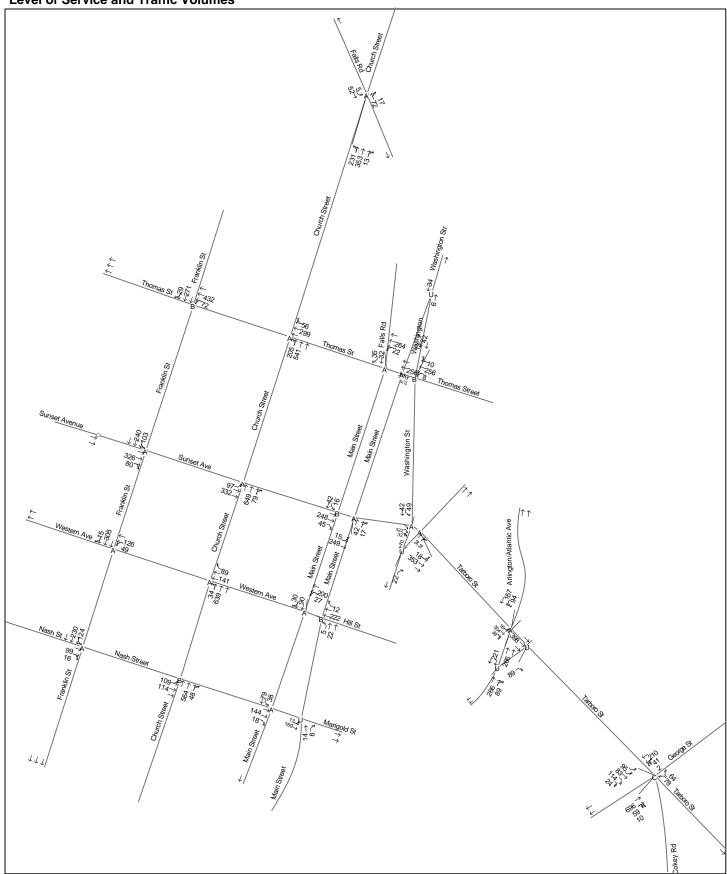
	Network Vehicle Delay (mph) [1]					
Alternative	A.M. Peak	Rank	P.M. Peak	Rank	Average Rank	Overall Rank
No-Build Conditions	14	-	13	-	-	-
Alternative 1	11	1	10	2	1.5	2
Alternative 2	9	5	9	3	4	3
Alternative 3	11	1	12	1	1	1
Alternative 4	10	4	8	4	4	3
Alternative 5	11	1	6	5	3	3

Note: Alternatives are ranked 1 through 5, highest average speed to lowest average speed.

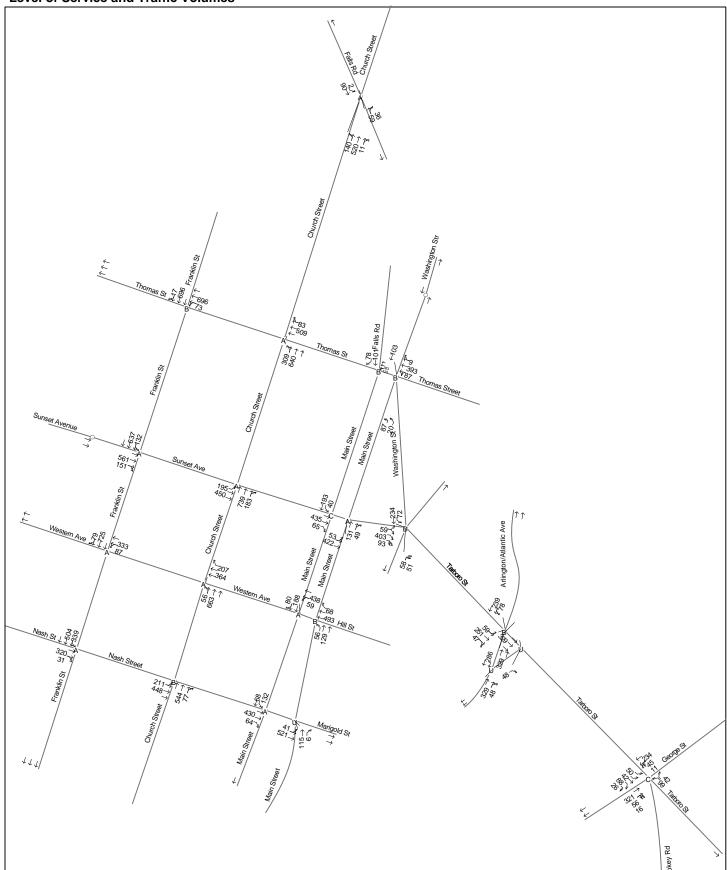
^[1] Summarizes average speed for all vehicles in the study area.



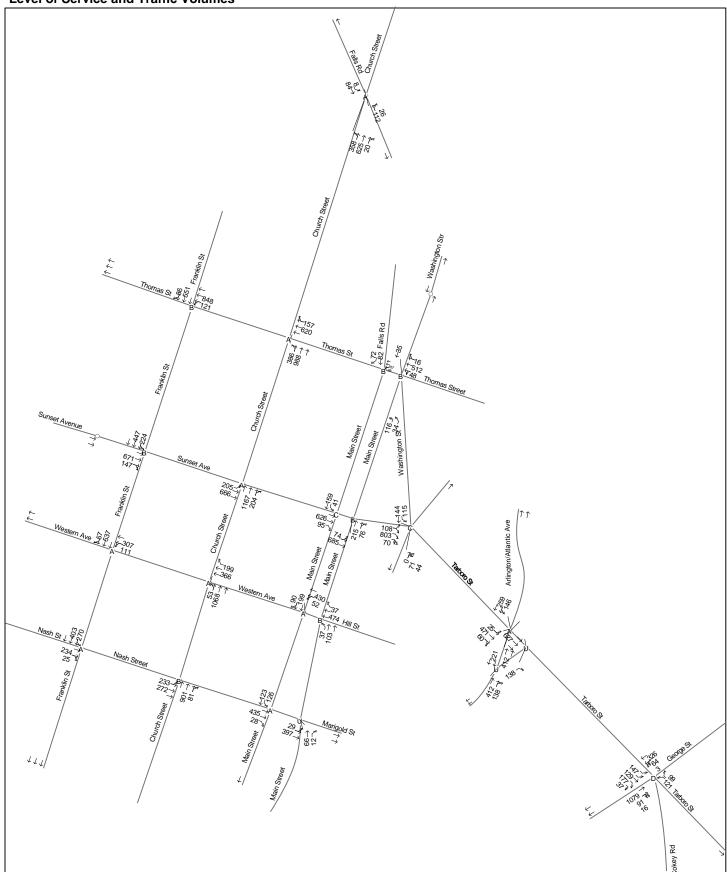
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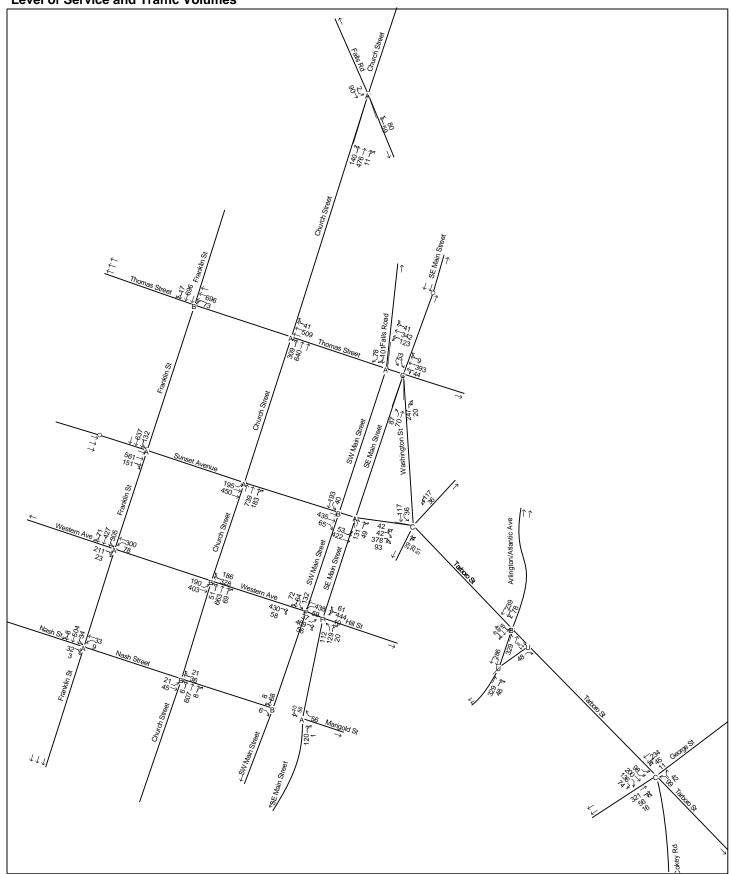
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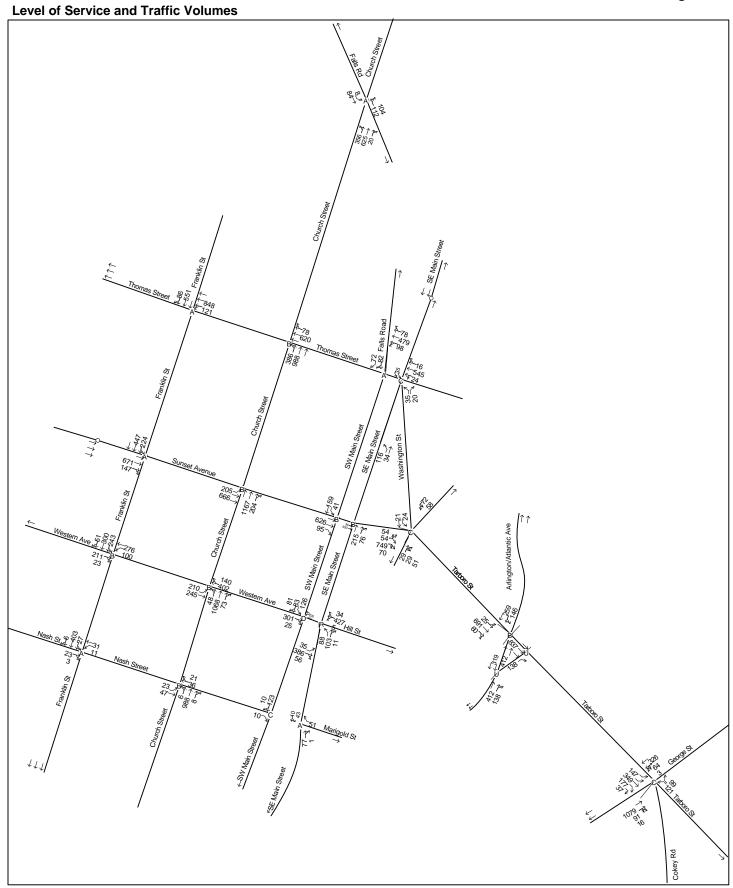
Rocky Mount Downtown Circulation Study



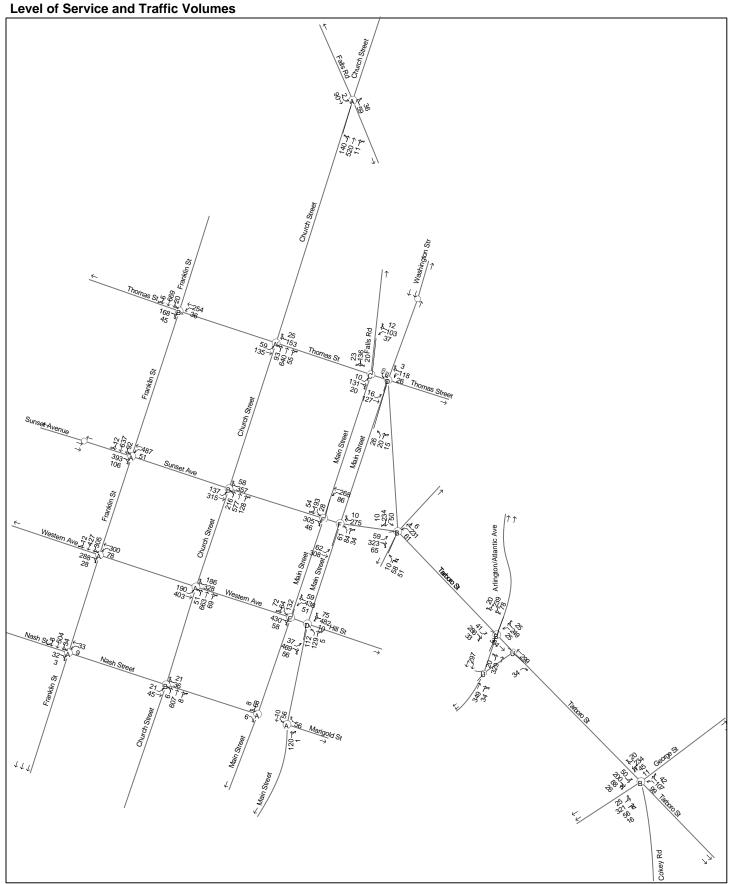
Rocky Mount Downtown Circulation Study



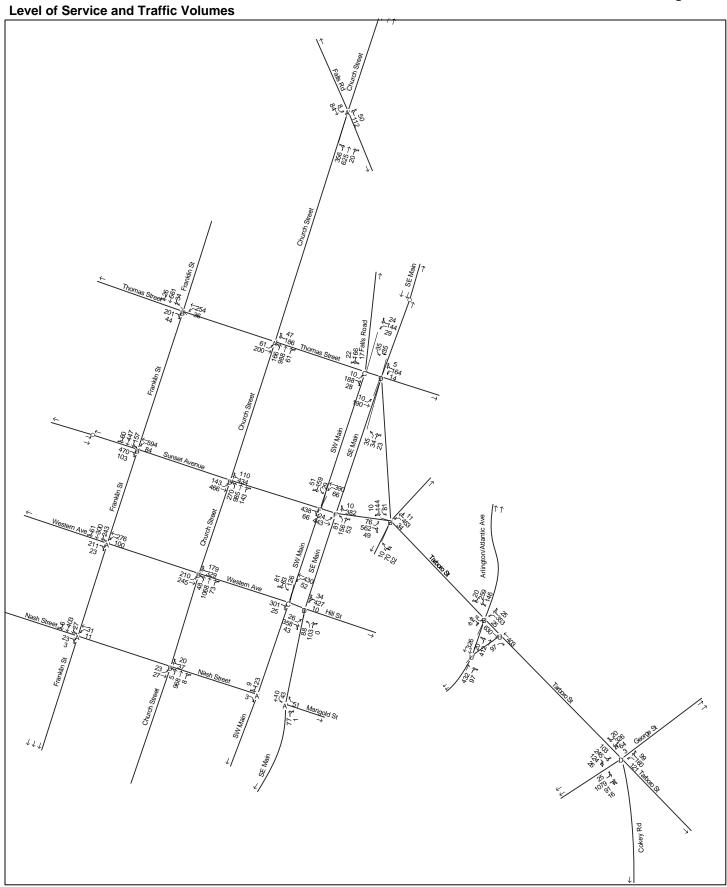
Rocky Mount Downtown Circulation Study



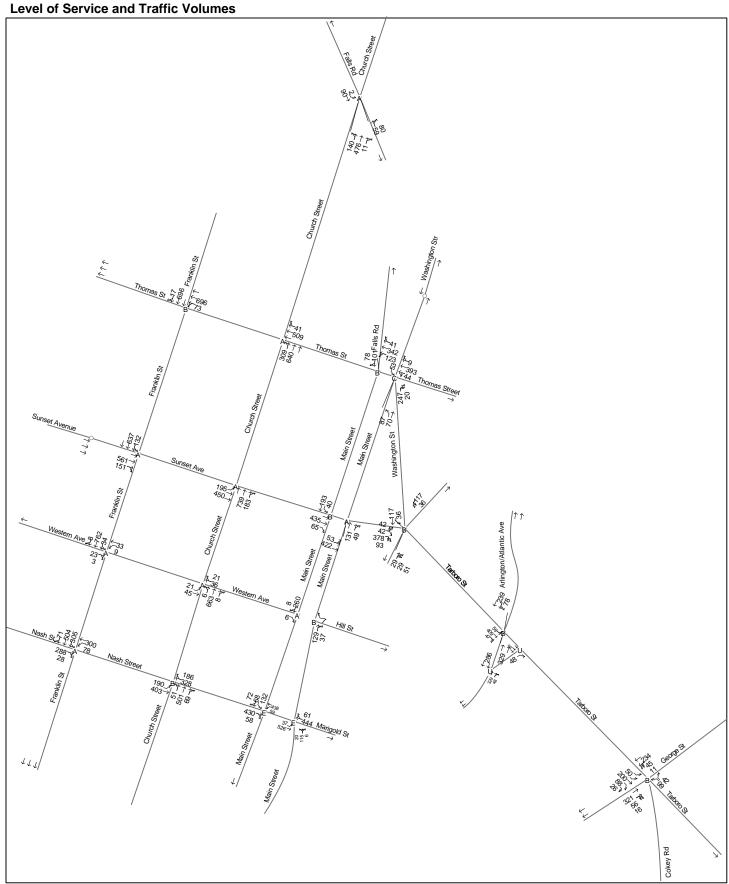
Rocky Mount Downtown Circulation Study



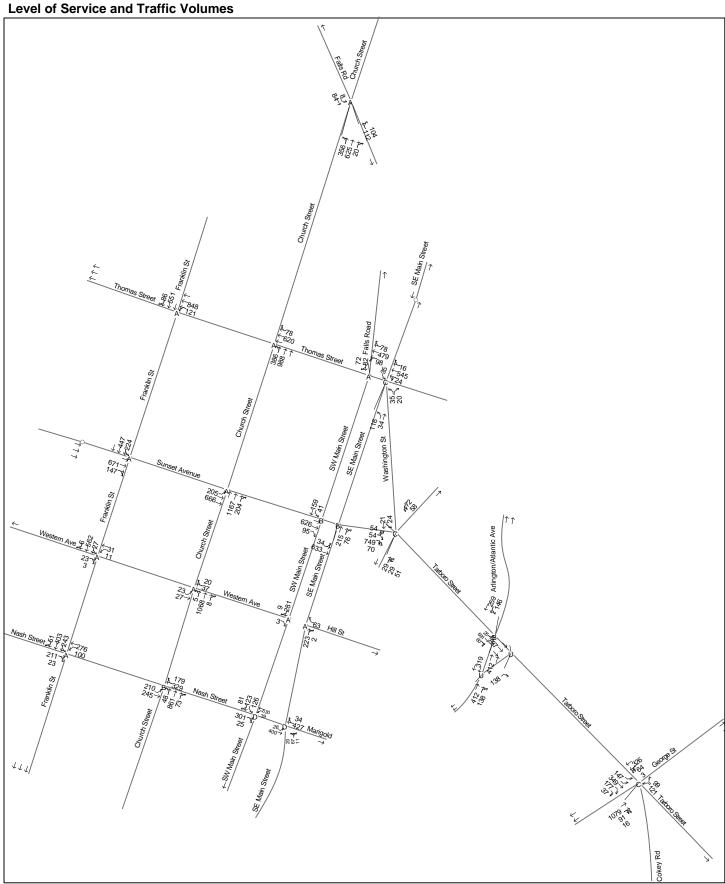
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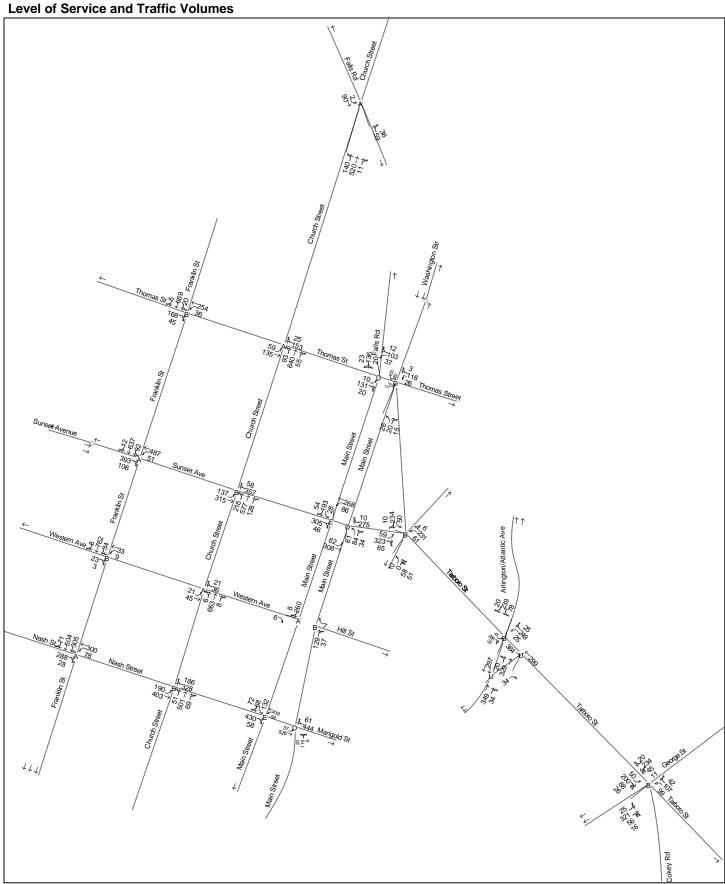
Rocky Mount Downtown Circulation Study



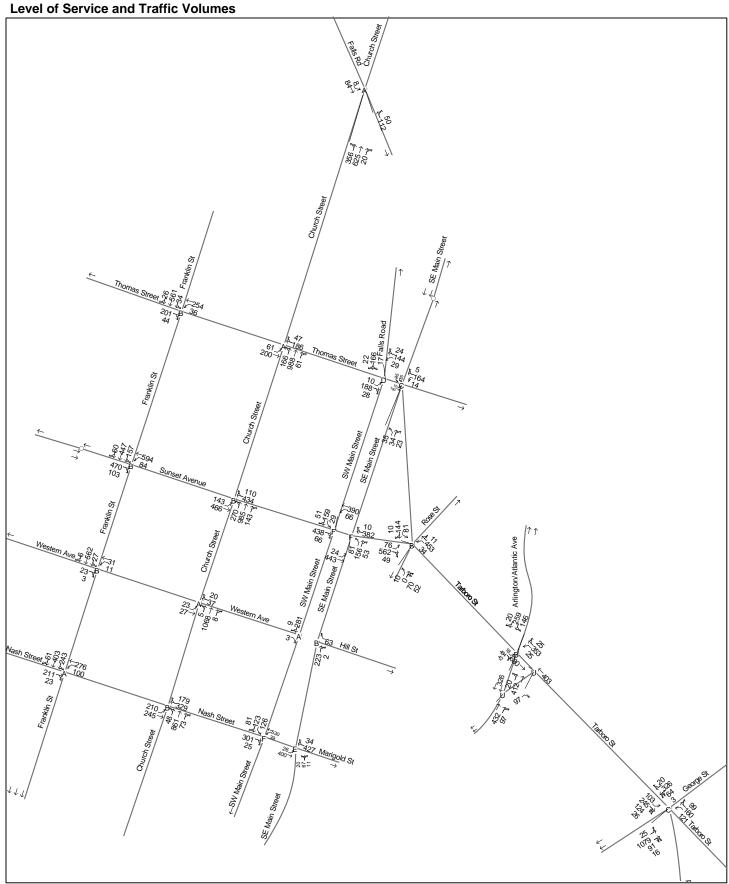
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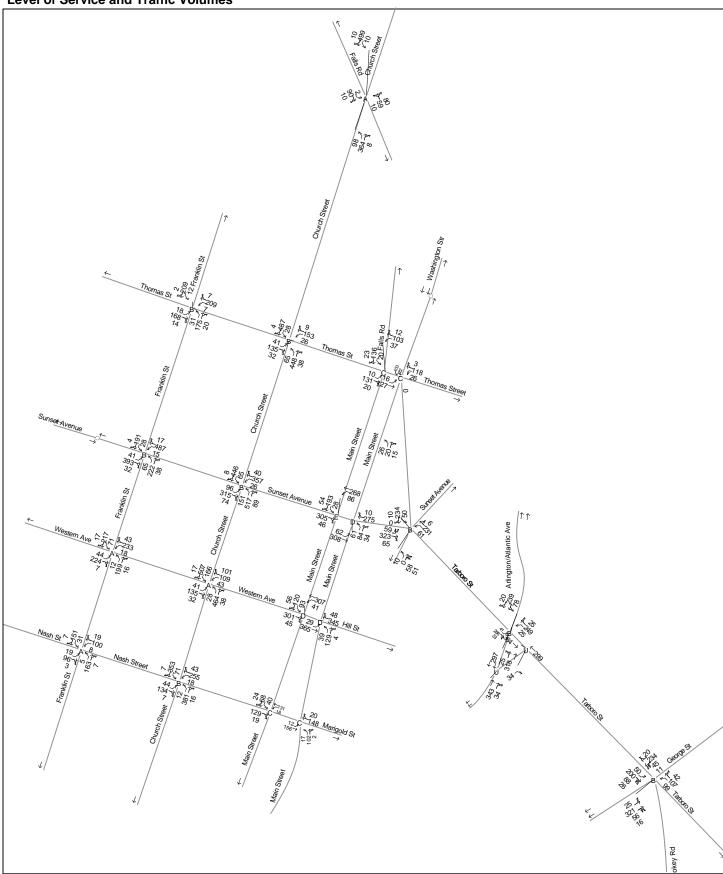
Rocky Mount Downtown Circulation Study



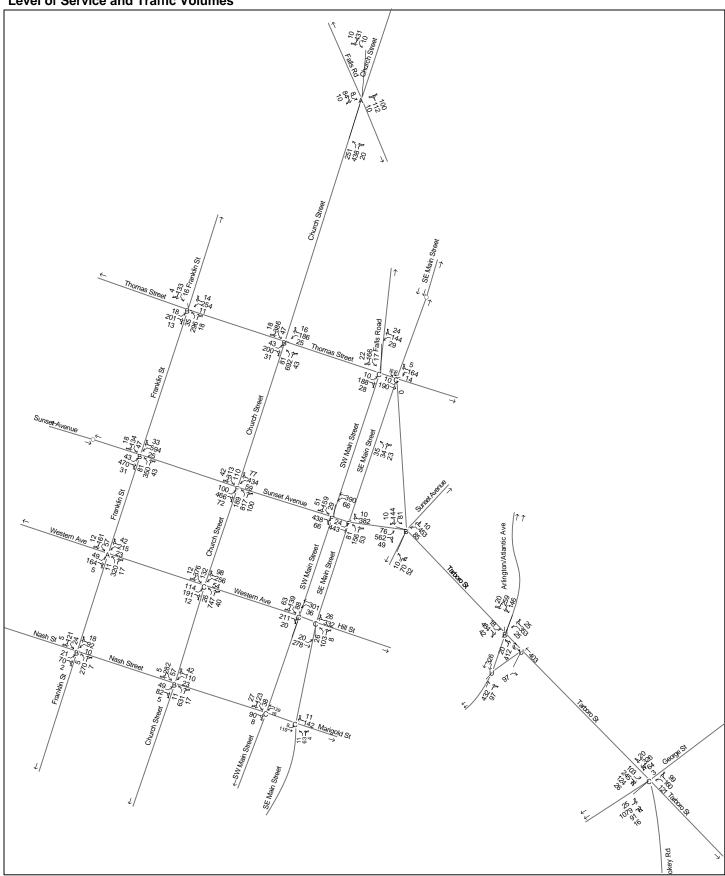
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6.0 OTHER SYSTEM MEASURES

P.M. PEAK HOUR TRAVEL TIME

Travel time is perhaps the most basic measure of the performance of a transportation network. Although travel time data collection was not part of this project, using the Synchro analysis output, it is possible to perform a comparative analysis of estimated travel time for a set origins and destinations. P.M. peak hour travel times were estimated for each alternative using the origins and destinations identified below.

- o Origins
 - Intersection of Sunset Avenue and Grace Street
 - o Intersection of Thomas Street and Atlantic Avenue
- Destinations
 - Intersection of Nash Street and S.W. Main Street
 - Intersection of Hill Street and S.E. Main Street
 - Intersection of Marigold Street and S.E. Main Street

Table 8 ranks Alternatives 1 through 5 based on the estimated travel time during the P.M. peak hour for each set of origins and destinations. The basis for the estimated travel times is summarized in Table 11 and Table 12.

Table 8 Alternatives Ranked by Estimated P.M. Peak Travel Times

		No- Build	Alt	:. 1	Alt	. 2	Alt	. 3	Alt	:. 4	Alt	. 5
Origin	Destination	Travel	Travel Time	Rank	Travel Time	Rank	Travel Time	Rank	Travel Time	Rank	Travel Time	Rank
Sunset	Nash Street at Main Street	3:02	2:33	1	2:53	2	3:03	3	3:22	4	3:53	5
Avenue at Grace	Hill Street at Main Street	3:47	1:59	1	4:22	3	3:42	2	5:46	5	5:31	4
Street	Marigold Street at Main Street	3:06	2:25	1	4:48	3	3:13	2	5:03	5	6:36	4
Thomas	Nash Street at Main Street	3:05	4:16	2	5:26	3	3:07	1	6:28	4	6:31	5
Street at Atlantic	Hill Street at Main Street	3:51	3:42	1	5:11	3	3:46	2	7:21	5	5:38	4
Avenue	Marigold Street at Main Street	3:10	4:08	2	5:36	3	3:17	1	6:37	5	5:45	4
Ave	rage Rank	-	1.	.6	2.	.8	1.	.8	4.	.6	4.	.3
Ov	erall Rank	-	1	ı	3	3	2	2	ţ	5	4	ı

Note: Alternatives are ranked 1 through 5, least travel time to most travel time.

In addition to estimating P.M. peak hour travel times for each alternative for the origins and destinations identified above, P.M. peak hour travel times were also estimated for emergency vehicles responding from Fire Station 1 to the intersection of Thomas Street and Church Street. Table 9 ranks the alternatives in terms of estimated P.M. peak hour travel times between Fire Station 1 and the intersection of Thomas Street and Church Street. The basis for the estimated travel times is shown in Table 13.

Table 9 Alternatives Ranked by Fire Station 1 P.M. Peak Hour Travel Times

Origin	Destination	No- Build	Alt. 1	Alt. 2	Alt. 3	Alt. 4	Alt. 5
Fire Station 1	Thomas Street at Church Street	2:42	4:28	4:11	5:11	6:07	3:48
	Rank	-	3	2	4	5	1

Note: Alternatives are ranked 1 through 5, least travel time to most travel time.

TRAFFIC QUEUES

One of the major concerns of stakeholders in the downtown area is the traffic queues expected to occur when the east-west one-way streets are converted to two-way operation. These concerns are magnified because of the combined impact of the traffic queues created when trains are passing through downtown temporarily stopping traffic from passing through downtown. The *SimTraffic* analysis outputs confirm that due to the reduced capacity of the intersections at the railroad crossings, traffic queues on the east-west streets are expected to increase when the streets are converted to two-way operation. Table 9 shows the estimated P.M. peak hour traffic queues for the east-west street on the approaches to Main Street. The alternatives were then ranked based on average estimated traffic queues by direction.

Table 10 Estimated Average P.M. Peak Hour Traffic Queues at Main Street

Street	No-E	Build	Altern	ative 1	Altern	ative 2	Altern	ative 3	Altern	ative 4	Altern	ative 5
Street	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
Thomas St [1]	-	115'	-	75'	25'	200'	-	115'	160'	235'	100'	120'
Sunset Ave/Tarboro St [1]	180'	-	55'	-	340'	210'	100'	-	385'	205'	345'	235'
Western Ave/Hill St [1]	-	145'	385'	240'	220'	280'	-	-	-	-	100'	245'
Nash St/Marigold St [1]	55'	-	•	-	ı	ı	305'	160'	285'	175'	75'	75'
Average Queue [1]	118'	130'	220'	158'	195'	230'	203'	138'	277'	205'	155'	169'
Rank by Direction	•	-	4	2	2	5	3	1	5	4	1	3
Average Rank [2]	•	•	3	3	3	.5		1	4	.5		2
Overall Rank [2]	•	•	2	2	;	3	2	1	Ę	5	,	1

Notes: [1] The average block lengths along the east-west streets are 400' for the eastbound streets and 270' for the westbound streets.

[2] Alternatives are ranked 1 through 5, shortest queues to longest queues

As indicated in Table 8, the impact of converting the one-way streets to two-way operation significantly increased traffic queue lengths on the approaches to Main Street and the railroad crossings.

TRAFFIC CIRCULATION IMPACTS

In addition to the level of service results and estimated travel time impacts, a number of general observations can be made concerning the anticipated impact of the various alternatives on traffic circulation in the downtown area.

Both of the railroad crossings under consideration for closure will significantly impact traffic circulation in the downtown core. Closing either of the railroad crossings under consideration would require the conversion of Hill Street/Western Avenue and Nash Street/Marigold Street to two-way operation and eliminate one of only four existing signalized railroad crossings in the downtown area. As illustrated in the level of service results, closing the rail crossings in the downtown area would be expected to significantly disrupt existing traffic patterns and reduce mobility and accessibility in the downtown area. It may be desirable to convert a block of Main Street to two-way operation between in order



Closing the Hill Street/Marigold Street railroad crossing will interrupt the counterclockwise traffic pattern downtown.

Street to two-way operation between in order to provide an acceptable level of accessibility.

Closing the railroad crossing on Hill Street/Western Avenue would have less impact on the counterclockwise traffic circulation in the downtown than will closing the rail crossing on Nash Street/Marigold Street. If the rail crossing on Hill Street/Western Avenue were closed the existing counterclockwise circulation pattern would remain intact. However, if the Nash Street/Marigold Street rail crossing is closed, the existing counterclockwise circulation pattern will be interrupted and shortened by a block, significantly impact the accessibility of the land uses on the block of Main Street between Hill Street/Western Avenue and Nash Street/Marigold Street.

In addition to vehicular traffic pattern impacts, it is important to recognize that the proposed changes to vehicular traffic patterns will also impact pedestrian activity and travel patterns in the downtown area. Converting one-way streets to two-way streets increases the number of possible pedestrian-vehicle conflicts at intersections. However, two-way street should decrease vehicle speeds. Closing the railroad crossings in the downtown area will also impact pedestrians. Although a railroad crossing may be closed to vehicular traffic, it is vital that all crossings remain open to pedestrian traffic.

Table 11 Estimated P.M. Peak Hour Travel Times – Sunset Avenue/Grace Street

		No-Build		Alternative 1		Alternative	2
Origin	Destination	Route	Travel Time [1]	Route	Travel Time [1]	Route	Travel Time [1]
	Nash Street at Main Street	Sunset -> Main	3:02	Sunset -> Main	2:33	Sunset -> Main	4:38
	Nasii Stieet at Maiii Stieet	Sunset -> Main		Suriset -> Mairi	2.33	Franklin -> Western -> SW Main	2:53
	Hill Street at Main Street	Sunset -> Main -> Nash -> SE Main	3:47	Sunset -> Main -> Western	1:59	Sunset -> Main -> Western	4:22
	Marigold Street at Main Street	Sunset -> Main -> Marigold	3:06	Sunset -> SW Main -> Western -> SE Main	2:25	Sunset -> SW Main -> Western -> SE Main	4:48
Sunset Avenue at Grace Street	Destination	Alternative 3		Alternative 4		Alternative 5	
	Nash Street at Main Street	Sunset -> Main	3:03	Sunset -> Main	4:53	Sunset -> Main	6:23
	Nasii Stieet at Maiii Stieet	Sunset -> Main	3.03	Franklin -> Western -> SW Main	3:22	Franklin -> Western -> SW Main	3:53
	Hill Street at Main Street	Sunset -> Main -> Nash -> SE Main	3:42	Sunset -> Main -> Nash -> SE Main	5:46	Sunset -> Main -> Western	5:31
	Marigold Street at Main Street	Sunset -> Main -> Nash	3:13	Sunset -> Main -> Nash	5:03	Sunset -> Main -> Nash	6:36

Table 12 Estimated P.M. Peak Hour Travel Times – Thomas Street/Church Street

		No-Build		Alternative 1	1	Alternative	2
Origin	Destination	Route	Travel Time [1]	Route	Travel Time [1]	Route	Travel Time [1]
	299 SW Main Nash Street @ S.W. Main Street	Sunset -> Main	3:05	Thomas -> Main	4:16	Thomas -> Main	5:26
	199 SE Main Hill Street at S.E. Main Street	Sunset -> Main -> Nash -> SE Main	3:51	Thomas -> Main -> Western	3:42	Thomas -> Main -> Western	5:11
Thomas	299 SE Main Marigold Street at S.E. Main Street	Sunset -> Main -> Marigold	3:10	Thomas -> Main -> Western -> SE Main	4:08	Thomas -> Main -> Western -> SE Main	5:36
Street at Atlantic	Destination	Alternative 3		Alternative 4		Alternative	5
Avenue	299 SW Main Nash Street @ S.W. Main Street	Thomas -> Main	3:07	Thomas -> Main	6:28	Thomas -> Main	6:31
	199 SE Main Hill Street at S.E. Main Street	Thomas -> Main -> Nash -> SE Main	3:46	Thomas -> Main -> Nash -> SE Main	7:21	Thomas -> Main -> Western	5:38
	299 SE Main Marigold Street at S.E. Main Street	Thomas -> Main -> Nash	3:17	Thomas -> Main -> Nash	6:37	Thomas -> Main -> Nash	5:45

Table 13 Estimated P.M. Peak Hour Travel Times – Fire Station 1

		No-Build	Alternative 1		Alternative 2		
Origin	Destination	Route	Travel Time [1]	Route	Travel Time [1]	Route	Travel Time [1]
	George -> Hill/Western -> Church 2:42		George -> Hill/Western - > Church	4:28	George -> Hill/Western -> Church	4:11	
Fire Station 1 Thomas Street at Church Street		Alternative 3	Alternative 4		Alternative	: 5	
		George -> Hill/Western -> Church	5:11	George -> Marigold/Nash -> Church	6:07	George -> Hill/Western -> Church	3:48

7.0 ECONOMIC IMPACTS

Analyses were performed to assess the relative impact of the alternatives on the economic development in the downtown area.

ONE-WAY STREET CONVERSIONS

Since World War II, the traditional response to increasing congestion levels through urban areas has been to improve the flow of motor vehicles by adopting one-way street operations. This strategy was designed to reduce congestion without having to widen streets or construct new facilities. From a traffic engineering perspective, the one-way street patterns cut down on pedestrian accidents, moved commuter traffic through downtowns more quickly, and allowed for better synchronization of traffic lights.

After 40-50 years, the majority of opinion and research now suggests that one-way streets may be more detrimental to downtowns than helpful. Studies suggest that one-way streets confuse and disorient visitors, speed traffic, intimidate pedestrians, and complicate access to businesses. Although they may enhance traffic flow through a retail area, one-way streets may also impede or discourage destination traffic to that retail area.

In the retail marketplace, they may diminish the attractiveness and value of properties that lose storefront exposure and evening peak traffic. Retail exposure and visibility is significantly reduced when one direction of traffic is removed, causing one side of cross streets to be effectively "eclipsed" from view. Since many shopping trips are conducted on the way home from work, retailers on one-way streets en route to employment centers could be negatively impacted.

During the 1990s, a national trend began to convert some one-way streets back to two-way traffic circulation systems. Since the late 1990s, a number of cities including Albuquerque, Austin, Chattanooga, Cincinnati, Des Moines, Portland (OR), Sacramento, Seattle, St. Paul, St. Petersburg, Tampa, and Toledo have either initiated or considered the conversion of their downtown one-way streets to two-way operations.

For example, since 1991, Lakeland, FL has completed two-way conversions on a number of one-way road pairs in its downtown area for the purpose of revitalizing the downtown core. In addition to increasing the visibility of retailers and restaurants, the strategy has helped slow traffic, enhance the pedestrian environment, and ease parking. Congestion increases and level of services (LOS) diminutions have been partly mitigated by the redistribution of traffic on the more highly connected road network.

RAIL CROSSING CLOSURES

In addition to posing serious threats to health and safety, urban at-grade railroad crossings can adversely impact a retail environment. Train-vehicle collisions not only result in death and injury, but also may cause destruction of property, fires, and explosions. These threats and the prospect of traffic delays may create significant physical and psychological barriers for potential retail visitors. By closing these crossings to vehicular traffic, the perception of safety and mobility within urban retail areas may be enhanced.

PEDESTRIAN SKETCH PLAN METHODOLOGY

A pedestrian sketch-plan is a method to estimate pedestrian volumes and subsequent retail sales impact under existing and future conditions in a retail pedestrian activity area. This tool is used to identify areas of pedestrian traffic based on existing traffic data without the need to conduct pedestrian counts on all facilities. In the absence of a comprehensive retail market analysis and study, a sketch-plan can also be used to forecast impacts to pedestrian volumes and retail traffic as a result of future land use and/or transportation trip generation changes.

Retail Traffic

In developing a model estimating the number of retail pedestrians on Main Street, the following assumptions were made based on our understanding of the downtown Rocky Mount area, and existing industry standards, and fitting or calibrating the sketch plan model to the results of our retail market overview:

- Commercial traffic (e.g. deliveries, business services), based on industry standards, is assumed to represent 5 percent of total traffic
- Destination traffic represents the number of vehicles on a purposeful trip (e.g. errand, going to work, going to residence, shopping)
- Given the existing retail environment and the business survey, and the calibration
 of traffic volumes to order of magnitude retail sales and future trends and
 scenarios, the retail capture rate of destination traffic is estimated to be 75
 percent on Main Street
- The average vehicle occupancy based on industry standards is estimated at 1.5 persons per vehicle
- Total retail pedestrian generation (vehicle) represents the number of retail pedestrians accessing the Rocky Mount study area
- Given the walkability of Main Street, retail walk trips are assumed to represent 10 percent of all retail vehicular trips

Based on peak hour turning movements, the estimated Average Daily Traffic (ADT) on Main Street in 2003 was 3,760. Of this daily total, 752 vehicles or 20 percent was assumed to be through traffic and 188 vehicles or 5 percent was assumed to be commercial traffic including deliveries and business services. Of the remaining destination traffic, 2,820 vehicles or 75 percent are assumed to be retail-oriented traffic.

Given average vehicle occupancy of 1.5, this traffic represents 3,173 retail pedestrians accessing Main Street by vehicle on a daily basis. Ten percent of this total approximates the number of additional retail pedestrians that access the study area through walk-only trips. In total, the vehicles and walkers traveling along Main Street represent approximately 3,490 retail pedestrians per day for the study area.

OVERVIEW

Using the pedestrian sketch-plan methodology, the existing retail traffic patterns were applied to the future traffic forecasts under the varying traffic assumptions for a 2025 design year. The following summarizes our analysis of the retail sales implications for the varying traffic alternatives.

- The primary impact of transportation alternatives is expected to be the redistribution of traffic within the downtown area
- Case studies suggest that retail destination traffic will sacrifice speed for an easier circulation pattern and an enhanced pedestrian environment
- Surveys of existing businesses indicate that most of their customers represent purposeful destination trips rather than traffic-related impulse or intercept trips



Main Street

- Given the nature of existing retail
 activity, other factors outside of traffic patterns including the pedestrian
 environment, available parking, retail marketing, merchandising programs,
 number and type of merchandise/services offered, competing retail, and the
 socioeconomic profiles of downtown residents, employees, and visitors are likely
 to have a larger impact on retail sales
- The psychological impact of increased congestion may reduce projected retail sales

CONCLUSIONS

- Other factors besides traffic will likely influence retail sales more than traffic modifications
- By diverting additional traffic onto Main Street, Alternative 3 and Alternative 4 could potentially generate higher design year retail sales than the No-Build scenario
- Alternatives 3 and 4 create a positive impact and are projected to exceed No-Build retail sales by \$7.8 million and \$8.8 million or 14.1% and 15.9% respectively (average annual change of 0.6% and 0.7% respectively)
- Alternatives 1, 2, and 5 are projected to generate fewer design year retail traffic and sales than the No-Build scenario decreasing sales by \$3.0 million, \$2.4 million, and \$3.6 million or 5.5%, 4.5%, and 6.5% respectively (average annual changes of 0.25%, 0.20%, and 0.29% respectively)
- All alternatives generate slightly more retail traffic than No-Build in the general area between the intersections of Main Street with Sunset Avenue and Hill Street/Western Avenue with Alternatives 3 and 4 generating moderately more retail traffic

- All alternatives generate slightly less retail traffic than No-Build in the general area between the intersections of Main Street with Thomas Street and Sunset Avenue
- Whereas Alternatives 3 and 4 generate slightly more retail traffic than No-Build along Main Street between Hill Street/Western Avenue and Nash Street/Marigold Street, Alternatives 1, 2, and 5 generate slightly less retail traffic

Table 14 Projected Retail Traffic and Sales

			Design Y	ear 2025		
	No-Build	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Average Daily Traffic (ADT)	12,090	11,430	11,550	13,800	14,010	11,310
Average Daily Retail Pedestrians	11,221	10,608	10,720	12,808	13,003	10,497
Annual Retail Pedestrians	1,851,470	1,750,397	1,768,774	2,113,341	2,145,500	1,732,020
Annual Retail Sales	\$55,544,105	\$52,511,920	\$53,063,227	\$63,400,219	\$64,365,005	\$51,960,614
Change from No-Build	-	(\$3,032,184)	(\$2,480,878)	\$7,856,114	\$8,820,900	(\$3,583,491)
Retail Sales Percent Change from No-Build	-	-5.5%	-4.5%	14.1%	15.9%	-6.5%
Annual Retail Sales Percent Change from No-Build, 2003-2025	-	-0.2%	-0.2%	0.6%	0.7%	-0.3%
Rank	-	4	3	2	1	5

Sources: BBP Associates, Martin/Alexiou/Bryson

Figure 20 2025 Projected Retail Sales by Transportation Alternative

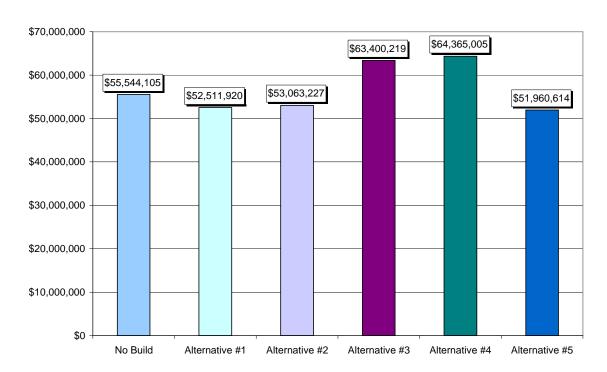


Table 15 Projected Change in Average Daily Traffic by Alternative

Main Street Segments	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Thomas - Sunset Southbound	0	430	0	430	430
Sunset - Thomas Northbound	-400	-990	-400	-990	-990
Thomas - Sunset Totals	-400	-560	-400	-560	-560
Percentage Change	-8.5%	-11.9%	-8.5%	-11.9%	-11.9%
Annual Percentage Change, 2003-2025	-0.4%	-0.5%	-0.4%	-0.5%	-0.5%
Sunset - Western Southbound	0	370	0	370	370
Western - Sunset Northbound	320	230	1,460	1,460	90
Sunset - Western Totals	320	600	1,460	1,830	460
Percentage Change	8.1%	15.2%	37.1%	46.4%	11.7%
Annual Percentage Change, 2003-2025	0.4%	0.7%	1.7%	2.1%	0.5%
Western - Nash Southbound	-910	-910	330	330	-560
Nash - Western Northbound	330	330	320	320	-120
Western - Nash Totals	-580	-580	650	650	-680
Percentage Change	-12.4%	-12.4%	13.9%	13.9%	-14.5%
Total Traffic	-660	-540	1,710	1,920	-780
Total Percentage Change	-5.5%	-4.5%	14.1%	15.9%	-6.5%
Annual Percentage Change, 2003-2025	-0.2%	-0.2%	0.6%	0.7%	-0.3%

Sources: Martin/Alexiou/Bryson, BBP Associates

Table 16 Retail Traffic Impact by Alternative

Main Street Intersection Areas	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Thomas - Sunset Totals	-	-	-	-	-
Sunset - Western Totals	+	+	++	++	+
Western - Nash Totals	-	-	+	+	-
Total Traffic	-	-	+	+	-

Sources: Martin/Alexiou/Bryson, BBP Associates

Slightly negative impact (-0.2% to -0.4% per year)

Slightly positive impact (0.4% to 0.7% per year)

Moderately positive impact (1.7% to 2.1% per year)

++

8.0 IMPLEMENTATION COSTS

In addition to evaluating the feasibility of closing either the Nash Street/Marigold Street or the Hill Street/Western Avenue at-grade railroad crossing and converting selecting one-way streets to two-way operation in terms of traffic operations and economic impact, the study also investigated the anticipated cost of the improvements required to implement the alternatives under evaluation. These preliminary cost estimates include items ranging from the cost of upgrading pavement markings to accommodate two-way traffic to realigning a portion of Sunset Avenue and Thomas Street in the vicinity of City Lake to facilitate the conversion of Sunset Avenue and Thomas Street to two-way operation. Cost estimates across the five (5) build alternative range from \$600,000 to \$2,300,000. More detail regarding the scope of the improvements associated with each of the build alternatives is provided below.

Table 17 Implementation Costs by Alternative

Alternative	Anticipated Implementation Cost	Rank
Alternative 1	\$670,000	2
Alternative 2	\$1,900,000	4
Alternative 3	\$600,000	1
Alternative 4	\$1,800,000	3
Alternative 5	\$2,300,000	5

COSTS BY ALTERNATIVE

Alternative 1

It is anticipated that the improvements required to accommodate the closure of the Nash Street/Marigold Street crossing and the conversion of the Hill Street/Western Avenue crossing to two-way operation would be on the order of \$670,000. In terms of specific improvements, the list includes:

- Remove the traffic signal and railroad crossing devices at the intersection of Nash Street/Marigold Street and Main Street,
- Construct a pedestrian crossing at the Nash Street/Marigold Street crossing to maintain pedestrian access between SW Main Street and SE Main Street,
- Upgrade 12 signalized intersections along Nash Street/Marigold Street and Hill Street/Western Avenue between George Street and Grace Street to accommodate two-way traffic,
- Upgrade the railroad protection devices (gates and flashers) at Hill Street/Western Avenue to accommodate two-way traffic,
- Remove the traffic signal at Marigold Street and Washington Street,
- Reconstruct the intersection of George Street and Hill Street to increase the turning radius for fire trucks turning right from George Street onto Hill Street,
- Install pavement markings along Nash Street/Marigold Street and Hill Street/Western Avenue and upgrade pavement markings at signalized intersections from George Street to Grace Street in order to accommodate twoway traffic.

Alternative 2

In addition to the improvements associated with Alternative 1, Alternative 2 includes the improvements required to accommodate the conversion of the Sunset Avenue/Tarboro Street and Thomas Street to two-way operation between City Lake and George Street. A preliminary sketch of the realignment of Sunset Avenue, Thomas Street, and Piedmont Avenue is included in Appendix F as Figures F-1 and F-2. Based on preliminary estimates, the cost associated with implementation of Alternative 2 is estimated to be on the order of \$1,900,000. In addition to the improvements associated with Alternative 1, the list of improvements required to implement this alternative include:

- Upgrade an additional 14 signalized intersections along Sunset Avenue/Tarboro Street and Thomas Street George Street and Lee Street to accommodate twoway traffic (this brings the total number of intersection upgrades to 26),
- Upgrade the railroad protection devices (gates and flashers) at Sunset Avenue/Tarboro Street and Thomas Street to accommodate two-way traffic,
- Remove the traffic signals along West Thomas Street at Pine Street and at Lee Street.
- Install pavement markings along Sunset Avenue/Tarboro Street and Thomas Street and upgrade pavement markings at signalized intersections from west of City Lake to George Street,
- Realign Sunset Avenue between River Drive and Piedmont Avenue, realign West Thomas Street to a "T-intersection" with Sunset Avenue just west of the BP Gas Station, and realign Piedmont Avenue at its intersection with Sunset Avenue.

Alternative 3

It is anticipated that the improvements required to accommodate the closure of the Hill Street/Western Avenue crossing and the conversion of the Nash Street/Marigold Street crossing to two-way operation would be on the order of \$600,000. In terms of specific improvements, the list includes:

- Remove the traffic signal and railroad crossing devices at the intersection Main Street and Hill Street/Western Avenue,
- Construct a pedestrian crossing at the Hill Street/Western Avenue crossing to maintain pedestrian access between SW Main Street and SE Main Street,
- Upgrade 12 signalized intersections along Nash Street/Marigold Street and Hill Street/Western Avenue between George Street and Grace Street to accommodate two-way traffic,
- Upgrade the railroad protection devices (gates and flashers) at Nash Street/Marigold Street to accommodate two-way traffic,
- Remove the traffic signal at Hill Street and Washington Street,
- Reconstruct the intersection of George Street and Marigold Street to increase the turning radius for fire trucks turning right from George Street onto Marigold Street.
- Install pavement markings along Nash Street/Marigold Street and Hill Street/Western Avenue and upgrade pavement markings at signalized intersections from George Street to Grace Street in order to accommodate twoway traffic.

Alternative 4

In addition to the improvements associated with Alternative 3, Alternative 4 includes the improvements required to accommodate the conversion of the Sunset Avenue/Tarboro Street and Thomas Street to two-way operation between City Lake and George Street.

Based on preliminary estimates, the cost associated with implementation of Alternative 2 is estimated to be on the order of \$1,800,000. In addition to the improvements associated with Alternative 3, the list of improvements required to implement this alternative include:

- Upgrade an additional 14 signalized intersections along Sunset Avenue/Tarboro Street and Thomas Street George Street and Lee Street to accommodate twoway traffic (this brings the total number of intersection upgrades to 26),
- Upgrade the railroad protection devices (gates and flashers) at Sunset Avenue/Tarboro Street and Thomas Street to accommodate two-way traffic,
- Remove the traffic signals along West Thomas Street at Pine Street and at Lee Street.
- Install pavement markings along Sunset Avenue/Tarboro Street and Thomas Street and upgrade pavement markings at signalized intersections from west of City Lake to George Street,
- Realign Sunset Avenue between River Drive and Piedmont Avenue, realign West Thomas Street to a "T-intersection" with Sunset Avenue just west of the BP Gas Station, and realign Piedmont Avenue at its intersection with Sunset Avenue.

Alternative 5

The improvements required to convert the Nash Street/Marigold Street – Hill Street/Western Avenue, Thomas Street-Sunset Avenue/Tarboro Street, and Church Street-Franklin Street one-way pairs to two-way operation, include a majority of the improvements associated with the four (4) build alternatives which included a rail crossing closure. Taking into account these improvements, as well as the changes required to accommodate the conversion of the Church Street-Franklin Street one-way pair to two-way operation, it is anticipated that the total cost to implement this alternative would be on the order of \$2,300,000. A preliminary sketch of the realignment of Franklin Street at North Church Street and the reconfiguration of the intersection of Franklin Street and Bassett Street is included in Appendix F as Figures F-3 and F-4.

In terms of specific improvements required to implement Alternative 5, the list includes:

- Upgrade 26 signalized intersections along Nash Street/Marigold Street, Hill Street/Western Avenue, Sunset Avenue/Tarboro Street, and Thomas Street between George Street and Lee Street to accommodate two-way traffic,
- Upgrade the railroad protection devices (gates and flashers) at all four at-grade railroad crossings downtown (Thomas Street to Nash Street/Marigold Street) to accommodate two-way traffic,
- Remove the traffic signals along West Thomas Street at Pine Street and at Lee Street.
- Install pavement markings along Nash Street/Marigold Street, Hill Street/Western Avenue, Sunset Avenue/Tarboro Street, and Thomas Street and upgrade pavement markings at signalized intersections from west of City Lake to George Street,
- Reconstruct the intersection of George Street and Hill Street to increase the turning radius for fire trucks turning right from George Street onto Hill Street,
- Realign Sunset Avenue between River Drive and Piedmont Avenue, realign West Thomas Street to a T-intersection with Sunset Avenue just west of the Kangaroo Express BP Gas Station, and realign Piedmont Avenue at its intersection with Sunset Avenue.

- Realign Franklin Street at its intersection with North Church Street to create a "T-intersection",
- Reconstruct intersection of Franklin Street and Bassett Street to accommodate two-way traffic on Franklin Street north of Bassett Street.
- Install pavement markings along Church Street and along Franklin Street and upgrade pavement markings at signalized intersections from the intersection of Franklin Street and North Church Street to Bassett Street.

CONCLUSIONS

Because of the wider area of impact, Alternatives 2, 4, and 5 have much higher implementation costs than Alternatives 1 and 3. Overall, Alternative 3 has the lowest implementation cost at \$600,000 followed by Alternative 1 at \$670,000.

9.0 PUBLIC PARTICIPATION

Public participation was a vital component of this project. Multiple opportunities were available for stakeholders to participate in the development of this project. As discussed in earlier sections of this report, stakeholders were interviewed as part of the economic analysis. Those interviews provided valuable insight into the history and future of business in the downtown area. In addition to the interviews, stakeholders were invited to two meetings to discuss the project. Stakeholders at both meetings were very involved and showed great interest in the direction of the project.

The first stakeholders meeting was held on November 4, 2004. Eleven stakeholders attended the meeting and topics of discussion included parking, the study area definition, future year traffic projections, the conversion of Sunset Avenue/Tarboro Street and Thomas Street to two-way operation, the inclusion of the railroad crossings on Goldleaf Street and Bassett Street, preferences to close the railroad crossing on Hill Street/Western Avenue instead of Nash Street/Marigold Street, and maintaining the counterclockwise traffic pattern on Main Street.

The project team presented the preliminary project work efforts to the planning board during their work session on November 9. Many of the same topics discussed at the stakeholder meeting on November 4 were discussed during the planning board work session. The topics of discussion included the purpose of the study, street cross-sections of streets if converted to two-way operation, anticipated economic impacts, possible changes to the railroad alignment, and difficulties downtown visitors have reported when traveling downtown. Based on feedback from this meeting, the project team began development of preliminary analysis scenarios.

Nine stakeholders attended a second stakeholders meeting on November 11, 2004. Many of the same topics discussed at the stakeholders meeting on November 4 were again discussed. The topics of discussion included the impacts of converting east-west streets to two-way operation with and without railroad closures, the impacts of closing railroad crossing in terms of traffic operations and economic development, possible conversion of Main Street to two-way operation, and the impact of converting Franklin Street and Church Street to two-way operation.

Based on the stakeholder and planning board meetings, the project team developed the five scenarios of future year conditions analyzed in this report.

On February 8, 2005, the project team attended a second planning board work session to present the preliminary analysis results. In addition to discussing the preliminary level of service results, board members were interested in the economic impacts of the proposed alternatives. The project team indicated that telephone interviews had been initiated in order to gather information directly from the stakeholders in the downtown area. Some members of the planning board again expressed concern that the scope of the project did not include the railroad crossings at Goldleaf Street and Bassett Street.

The feedback gathered at the stakeholder meetings provided a clear understanding of the local sentiment and provided direction for the development of the analysis scenarios included in this report. The active participation of the local stakeholders and planning board members ensured that the project continued to focus on issues that are important to the local community. Notes from the stakeholder and planning board meetings held during the development of this project are included in Appendix E.

10.0 FINDINGS AND RECOMMENDATIONS

LEVEL OF SERVICE ANALYSIS RESULTS

The level of service results indicate that the intersections adjacent to the railroad tracks (Main Street) are the most affected by the changes in traffic associated with the various alternatives. This is due to the geometry of the intersections that will result should the streets crossing the railroad be converted to two-way operation. Because of the location of the railroad crossings, the intersections adjacent to the railroad tracks are extremely wide. The location of the railroad tracks in close proximity to Main Street creates two very closely spaced intersections on each side of the railroad tracks that are operate as a single intersection. Fortunately, with the east-west streets being one-way, it is possible to provide an acceptable level of service. However, should the east-west streets be converted to two-way operation, the signal phasing would become much more complex. With such a wide intersection and the location of the railroad tracks, it is not feasible to allow permitted left-turn movements. Therefore, each approach to the intersection requires a protected phase within the traffic signal cycle. This increases the signal cycle length and increased delays for each approach to the intersection. This effect is apparent in the level of service results for Alternative 1 through 5. The following tables summarize the level of service analyses in terms of average vehicle delay and travel speeds for each alternative.

	Ave	rage Vehicle	Delay (sec/vel	1)		
Alternative	A.M. Peak	Rank	P.M. Peak	Rank	Average Rank	Overall Rank
No-Build Conditions	00:11	-	00:12	-	-	-
Alternative 1	00:17	1	00:19	2	1.5	2
Alternative 2	00:24	5	00:26	3	4	3
Alternative 3	00:17	1	00:14	1	1	1
Alternative 4	00:20	4	00:30	4	4	3
Alternative 5	00:19	3	00:42	5	4	3

Note: Alternatives are ranked 1 through 5, lowest average delay to highest average delay.

	Av	erage Vehic	e Speed (mph)			
Alternative	A.M. Peak	Rank	P.M. Peak	Rank	Average Rank	Overall Rank
No-Build Conditions	14	-	13	-	-	-
Alternative 1	11	1	10	2	1.5	2
Alternative 2	9	5	9	3	4	3
Alternative 3	11	1	12	1	1	1
Alternative 4	10	4	8	4	4	3
Alternative 5	11	1	6	5	3	3

Note: Alternatives are ranked 1 through 5, highest average speed to lowest average speed.

In terms of average vehicle delay and average vehicle travel speeds, Alternative 3 results in less delay and higher travel speeds in comparison to the other alternatives.

OTHER SYSTEM MEASURES

Analysis of P.M. Peak hour travel times for alternatives 1 through 5 indicate that number of streets that are converted to two-way operation increases so does the estimated travel time across the study area. As travel time increases through and about the study area, motorists, especially commuters may eventually find more convenient routes through downtown or may divert around downtown all together. The table below shows how the alternatives rank relative to one another based on the estimated P.M. peak hour travel times for the origins and destinations shown.

Alternative Ranking by P.M. Peak Hour Travel Time

		No- Build	Alt	:. 1	Alt	. 2	Alt	3	Alt	:. 4	Alt	i. 5
Origin	Destination	Travel	Travel Time	Rank								
Sunset	Nash Street at Main Street	3:02	2:33	1	2:53	2	3:03	3	3:22	4	3:53	5
Avenue at Grace	Hill Street at Main Street	3:47	1:59	1	4:22	3	3:42	2	5:46	5	5:31	4
Street	Marigold Street at Main Street	3:06	2:25	1	4:48	3	3:13	2	5:03	5	6:36	4
Thomas	Nash Street at Main Street	3:05	4:16	2	5:26	3	3:07	1	6:28	4	6:31	5
Street at Atlantic	Hill Street at Main Street	3:51	3:42	1	5:11	3	3:46	2	7:21	5	5:38	4
Avenue	Avenue Marigold Street at Main Street		4:08	2	5:36	3	3:17	1	6:37	5	5:45	4
Average Rank		-	1.6 2		2.	.8	1.8		4.6		4.	.3
Overall Rank		-	1	1	3		2		5		4	

Note: Alternatives are ranked 1 through 5, least travel time to most travel time.

Emergency vehicles are affected by changes to the downtown street operations just as other vehicles traveling in the downtown area. Analysis of estimated peak hour travel times for emergency vehicles from Fire Station 1 to the intersection of Thomas Street and Church Street indicate that Alternatives 3 and 4 would have the most significant impact on travel time. The table below ranks the alternative relative to one another based on the estimated peak hour travel times between Fire Station 1 and the intersection of Thomas Street and Church Street. While the results indicate an increase in travel time, responding to the intersection of Thomas Street and Church Street is the worst-case scenario for Fire Station 1. The intersection of Thomas Street and Church Street is in the far reaches of the response area for Fire Station 1 and the travel times analyzed in this study are for the P.M. peak hour. Travel times during off-peak periods would not be expected to reach these levels.

Alternative Ranking by Fire Station 1 P.M. Peak Hour Travel Time

Origin	Destination	No- Build	Alt. 1	Alt. 2	Alt. 3	Alt. 4	Alt. 5
Fire Station 1	Thomas Street at Church Street	2:42	4:28	4:11	5:11	6:07	3:48
	Rank	-	3	2	4	5	1

Note: Alternatives are ranked 1 through 5, least travel time to most travel time.

Simtraffic analysis outputs indicate that the conversion of the east-west streets to two-way operation will result in significant increases in traffic queues on the approaches to Main Street. The table below shows the how the alternatives rank relative to one another based on estimated P.M. peak hour traffic queues.

Alternative Ranking by P.M. Peak Hour Traffic Queues at Main Street

Street	No-Build		Alternative 1		Altern	ative 2	Alterna	ative 3	Altern	ative 4	Alterna	ative 5
Street	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
Thomas St	-	115'	-	75'	25'	200'	•	115'	160'	235'	100'	120'
Sunset Ave/Tarboro St	180'	1	55'	-	340'	210'	100'	-	385'	205'	345'	235'
Western Ave/Hill St	-	145'	385'	240'	220'	280'	•	-	-	-	100'	245'
Nash St/Marigold St	55'		-	-	•		305'	160'	285'	175'	75'	75'
Average Queue	118'	130'	220'	158'	195'	230'	203'	138'	277'	205'	155'	169'
Rank by Direction	•	ı	4	2	2	5	3	1	5	4	1	3
Average Rank	•	•	3		3.5		4		4.5		2	
Overall Rank			2		3		4		5		1	

Notes: [1] The average block lengths along the east-west streets are 400' for the eastbound streets and 270' for the westbound streets.

[2] Alternatives are ranked 1 through 5, shortest queues to longest queues

Certain conclusions can also be drawn based the impact that closing a railroad crossing would have in the downtown area. Stakeholders have indicated a desire to maintain current counterclockwise traffic pattern on Main Street. Closing the railroad crossing Nash Street/Marigold Street will disrupt that traffic pattern. However, closing the rail crossing on Hill Street/Western Avenue allows the counterclockwise traffic pattern on Main Street between Thomas Street and Nash Street/Marigold Street to be maintained.

ECONOMIC IMPACTS

Analyses were also performed to assess the relative economic impact of each alternative. The analyses included interviewing the downtown stakeholders to assess the current business climate in the downtown area and to gauge the economic future of the downtown area. A model was developed to assess the relative economic impacts of each alternative based on the prevailing traffic patterns created by each alternative. The conclusions of the economic analysis is summarized below:

- Other factors besides traffic will likely influence retail sales more than traffic modifications
- By diverting additional traffic onto Main Street, Alternative 3 and Alternative 4 could potentially generate higher design year retail sales than the No-Build scenario
- Alternatives 3 and 4 create a positive impact and are projected to exceed No-Build retail sales by \$7.8 million and \$8.8 million or 14.1% and 15.9% respectively (average annual change of 0.6% and 0.7% respectively)
- Alternatives 1, 2, and 5 are projected to generate fewer design year retail traffic and sales than the No-Build scenario decreasing sales by \$3.0 million, \$2.4 million, and \$3.6 million or 5.5%, 4.5%, and 6.5% respectively (average annual changes of 0.25%, 0.20%, and 0.29% respectively)
- All alternatives generate slightly more retail traffic than No-Build in the general area between the intersections of Main Street with Sunset Avenue and Hill Street/Western Avenue with Alternatives 3 and 4 generating moderately more retail traffic
- All alternatives generate slightly less retail traffic than No-Build in the general area between the intersections of Main Street with Thomas Street and Sunset Avenue
- Whereas Alternatives 3 and 4 generate slightly more retail traffic than No-Build along Main Street between Hill Street/Western Avenue and Nash Street/Marigold Street, Alternatives 1, 2, and 5 generate slightly less retail traffic

The table below summarizes the impact of each alternative on retail sales in the downtown area.

	No-Build	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Annual Retail Sales	\$55.5 M	\$52.5 M	\$53.0 M	\$63.40 M	\$64.37 M	\$51.0
Rank	-	4	3	2	1	5

IMPLEMENTATION COSTS

In addition to evaluating the feasibility of closing either the Nash Street/Marigold Street or the Hill Street/Western Avenue at-grade railroad crossing and converting selecting one-way streets to two-way operation in terms of traffic operations and economic impact, the study also investigated the anticipated cost of the improvements required to implement the alternatives under evaluation. These preliminary cost estimates include items ranging from the cost of upgrading pavement markings to accommodate two-way

traffic to realigning a portion of Sunset Avenue and Thomas Street in the vicinity of City Lake to facilitate the conversion of Sunset Avenue and Thomas Street to two-way operation. Cost estimates across the five (5) build alternative range from \$600,000 to \$2,300,000. The table below includes a summary of the anticipated improvements associated with each of the alternatives. More detail regarding the scope of the improvements associated with each of the build alternatives is provided below. The table below ranks the alternatives relative to one another based on the anticipated implementation cost.

Alternative Ranking by Implementation Cost

Alternative	Anticipated Implementation Cost	Rank
Alternative 1	\$670,000	2
Alternative 2	\$1,900,000	4
Alternative 3	\$600,000	1
Alternative 4	\$1,800,000	3
Alternative 5	\$2,300,000	5

OVERALL ALTERNATIVE RANKING

A number of measures were developed in this study to assess the impact of each alternative on traffic operations in the downtown area. The alternative has been ranked relative to one based on each of these alternatives. The table below summarizes those rankings based on average vehicle delay, average vehicle speed, average P.M. peak hour traffic queues at Main Street, average P.M. peak hour travel time between specified origins, average P.M. peak hour travel times between Fire Station 1 and the intersection of Thomas Street and Church Street, anticipated Implementation Costs, and impact on retail traffic.

Overall Alternative Ranking

Alternative	Average Vehicle Delay	Average Vehicle Speed	Average P.M. Peak Hour Queues	Average P.M. Peak Hour Travel Times	Average Fire Station 1 P.M. Peak Hour Travel Times	Anticipated Implementation Cost	Impact on Retail Traffic	Average Ranking	Overall Ranking
1	2	2	2	1	3	2	4	2.3	2
2	3	3	3	3	2	4	3	3.0	3
3	1	1	4	2	4	1	2	2.1	1
4	3	3	5	5	5	3	1	3.6	5
5	3	3	1	4	1	5	5	3.1	4

RECOMMENDATIONS

This report includes a relatively detailed evaluation of the implications of closing one of the existing at-grade railroad crossings and the conversion of one-way streets to twoway operation in the downtown area. However, in addition to the impacts measured in this report, there are a wide range of other impacts that must factor into any decision concerning changes to traffic circulation in the downtown study area. With that said, the intent of this study is to quantify the impacts of the various alternatives under consideration in terms of traffic operations and economic impact and to compare the alternatives to one another based on those impacts.

The alternatives considered in this study result have varying impacts on traffic operations and the economic outlook in the downtown area. However, based on the results of the analyses in this study, Alternative 3 ranks the highest in comparison to all the other alternatives. If implemented, Alternative 3 would be expected to result in increased delay in the study area, but it is expected to have a positive impact on retail traffic in the downtown area and to have the lowest implementation cost of all the alternatives considered.

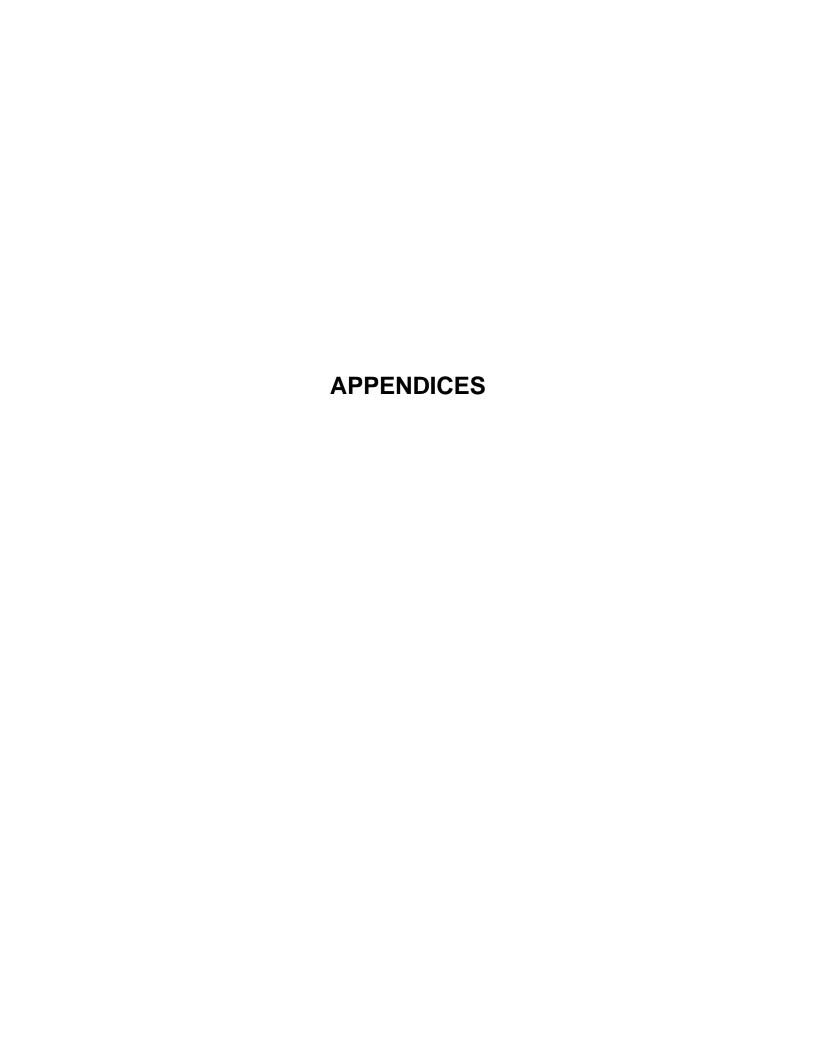
The improvements required to implement Alternative 3 are estimated to cost \$600,000. Alternative 3 requires the following improvements:

- Remove the traffic signal and railroad crossing devices at the intersection of Hill Street/Western Avenue,
- Construct a pedestrian crossing at the Hill Street/Western Avenue crossing to maintain pedestrian access between SW Main Street and SE Main Street,
- Upgrade 12 signalized intersections along Nash Street/Marigold Street and Hill Street/Western Avenue between George Street and Grace Street to accommodate two-way traffic,
- Upgrade the railroad protection devices (gates and flashers) at Nash Street/Marigold Street to accommodate two-way traffic,
- Remove the traffic signal at Hill Street and Washington Street,
- Reconstruct the intersection of George Street and Marigold Street to increase the turning radius for fire trucks turning right from George Street onto Marigold Street,
- Install pavement markings along Nash Street/Marigold Street and Hill Street/Western Avenue and upgrade pavement markings at signalized intersections from George Street to Grace Street in order to accommodate twoway traffic.

Due to the reduction in traffic on streets affect by the proposed closure of railroad crossings, it may be possible to remove the existing traffic signals at some intersections. Possible candidates for removal of traffic signals if the railroad crossings are closed include:

- Nash Street/Marigold Street at Main Street
- Marigold Street at Washington Street
- West Thomas Street at Pine Street and at Lee Street
- Hill Street/Western Avenue at Main Street

It is recommended, even though vehicular rail crossings may be closed, that pedestrian crossings of the railroad tracks be maintained and that the pedestrian crossings be designed to be safe and ADAA compliant.



APPENDIX A:

CSX TRAIN INTERRUPTIONS

Train Preemption Summary

Tuesday, March 1 thru Monday, March 7, 2005

Mai	n St. & Mar St./Nash St	0	Hill S	t./Western A Main St.	Ave. &	-	nin St. & Suve./Tarboro		Main S			
# of preempts	Day	Duration	# of preempts	Started	Duration	# of preempts	Started	Duration	# of preempts	Started	Duration	
38	Tuesday	1:37	38	Tuesday	1:58	38	Tuesday	1:47	35	Tuesday	1:42	7:04
43	Wednesday	2:14	43	Wednesday	2:28	43	Wednesday	2:29	41	Wednesday	2:18	9:29
49	Thursday	2:14	46	Thursday	2:36	46	Thursday	2:29	43	Thursday	2:30	9:49
42	Friday	1:59	49	Friday	2:21	41	Friday	2:04	37	Friday	2:01	8:25
42	Saturday	2:07	42	Saturday	2:24	42	Saturday	2:26	39	Saturday	2:25	9:22
40	Sunday	2:05	41	Sunday	2:20	39	Sunday	2:14	38	Sunday	2:06	8:45
36	Monday	1:42	36	Monday	1:56	36	Monday	1:53	34	Monday	1:48	7:19
				-								
290		13:58	295		16:03	285		15:22	267		14:50	60:13:00

Train Preemption

Tuesday, March 1, 2005

I	Main St./	. & Mar Nash St	_	Hi	ill St./W	estern A	Ave. &			St. & Su Farboro		Ma	ain St. & Wash		
#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration
1	01:37	01:40	0:03	1	01:37	01:40	0:03	1	01:37	01:40	0:03	1	01:37	01:39	0:02
2	02:09	02:13	0:04	2	02:08	02:13	0:05	2	02:08	02:13	0:05	2	02:09	02:13	0:04
3	03:59	04:02	0:03	3	03:58	04:02	0:04	3	03:58	04:02	0:04	3	03:58	04:02	0:04
4	04:42	04:44	0:02	4	04:42	04:44	0:02	4	04:42	04:44	0:02	4	04:42	04:45	0:03
5	04:52	05:00	0:08	5	04:52	05:00	0:08	5	04:52	05:00	0:08	5	04:53	05:01	0:08
6	05:19	05:21	0:02	6	05:19	05:21	0:02	6	05:19	05:21	0:02	6	05:19	05:21	0:02
7	05:25	05:26	0:01	7	05:25	05:26	0:01	7	05:26	05:26	0:00		00.17	00.21	0.02
8	05:28	05:29	0:01	8	05:28	05:30	0:02	8	05:28	05:30	0:02	7	05:28	05:30	0:02
9	06:24	06:27	0:03	9	06:23	06:27	0:04	9	06:24	06:27	0:03	8	06:24	06:27	0:03
10	06:53	06:55	0:02	10	06:52	06:55	0:03	10	06:53	06:55	0:02	9	06:53	06:55	0:02
11	08:03	08:06	0:03	11	08:03	08:06	0:03	11	08:03	08:07	0:04	10	08:04	08:07	0:03
12	09:51	09:55	0:04	12	09:49	09:55	0:06	12	09:49	09:55	0:06	11	09:48	09:54	0:06
13	10:27	10:28	0:01	13	10:27	10:28	0:01	13	10:27	10:28	0:01	12	10:27	10:28	0:01
14	10:38	10:39	0:01	14	10:37	10:39	0:02	14	10:38	10:39	0:01	13	10:38	10:39	0:01
15	10:54	10:55	0:01	15	10:53	10:55	0:02	15	10:53	10:54	0:01	14	10:52	10:54	0:02
16	11:08	11:10	0:02	16	11:08	11:11	0:03	16	11:09	11:11	0:02	15	11:09	11:11	0:02
17	12:11	12:18	0:07	17	12:09	12:17	0:08	17	12:09	12:16	0:07	16	12:08	12:15	0:07
18	13:17	13:21	0:04	18	13:16	13:21	0:05	18	13:16	13:22	0:06	17	13:18	13:22	0:04
19	13:30	13:31	0:01	19	13:30	13:31	0:01	19	13:30	13:31	0:01	- /	15.10	13.22	0.01
20	13:35	13:36	0:01	20	13:35	13:36	0:01	20	13:35	13:36	0:01	18	13:35	13:36	0:01
21	13:55	13:56	0:01	21	13:55	13:56	0:01	21	13:55	13:56	0:01	10	10.00	13.30	0.01
22	13:59	14:00	0:01	22	13:59	14:01	0:02	22	14:00	14:01	0:01	19	14:00	14:01	0:01
23	14:23	14:24	0:01	23	14:23	14:24	0:01	23	14:23	14:24	0:01	20	14:24	14:24	0:00
24	14:28	14:29	0:01	24	14:28	14:30	0:02	24	14:28	14:30	0:02	21	14:29	14:30	0:01
25	14:52	14:56	0:04	25	14:51	14:56	0:05	25	14:52	14:57	0:05	22	14:53	14:57	0:04
26	15:29	15:30	0:01	26	15:29	15:30	0:01	26	15:29	15:30	0:01	23	15:28	15:29	0:01
27	16:05	16:06	0:01	27	16:04	16:05	0:01	27	16:04	16:05	0:01	24	16:03	16:05	0:02
28	17:07	17:09	0:02	28	17:07	17:09	0:02	28	17:07	17:08	0:01	25	17:06	17:08	0:02
29	19:41	19:43	0:02	29	19:41	19:43	0:02	29	19:41	19:43	0:02	26	19:41	19:43	0:02
30	19:57	20:00	0:03	30	19:56	20:00	0:04	30	19:56	19:59	0:03	27	19:55	19:59	0:04
31	20:01	20:02	0:01	31	20:01	20:02	0:01	31	20:00	20:02	0:02	28	20:00	20:01	0:01
32	20:34	20:37	0:03	32	20:33	20:36	0:03	32	20:33	20:36	0:03	29	20:33	20:36	0:03
33	20:46	20:48	0:02	33	20:45	20:48	0:03	33	20:45	20:47	0:02	30	20:45	20:47	0:02
34	20:51	20:57	0:06	34	20:49	20:57	0:08	34	20:49	20:56	0:07	31	20:48	20:55	0:07
35	21:13	21:16	0:03	35	21:12	21:16	0:04	35	21:12	21:15	0:03	32	21:11	21:14	0:03
36	22:36	22:40	0:04	36	22:36	22:40	0:04	36	22:36	22:40	0:04	33	22:35	22:40	0:05
37	22:51	22:55	0:04	37	22:50	22:55	0:05	37	22:50	22:54	0:04	34	22:50	22:54	0:04
38	23:11	23:14	0:03	38	23:10	23:13	0:03	38	23:10	23:13	0:03	35	23:10	23:13	0:03
			1:37				1:58				1:47				1:42

Train Preemption

Wednesday, March 2, 2005

I	Main St./	. & Mar Nash St	_	H	ill St./W	estern A	Ave. &			St. & Su Farboro		Ma	nin St. & Wash	as St. & St.	
#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration
1	00:02	00:03	0:01	1	00:02	00:03	0:01	1	00:01	00:03	0:02	1	00:01	00:02	0:01
2	00:05	00:09	0:04	2	00:04	00:03	0:06	2	00:05	00:03	0:05	2	00:06	00:02	0:04
3	00:24	00:26	0:02	3	00:24	00:27	0:03	3	00:03	00:27	0:03	3	00:24	00:27	0:03
4	02:16	02:25	0:02	4	02:15	02:24	0:09	4	02:15	02:23	0:08	4	02:15	02:22	0:07
5	03:40	03:41	0:01	5	03:40	03:41	0:01	5	03:40	03:41	0:01	5	03:39	03:40	0:01
6	04:00	04:08	0:08	6	04:00	04:08	0:01	6	04:00	04:09	0:09	6	04:00	04:09	0:09
7	04:22	04:24	0:02	7	04:22	04:24	0:02	7	04:22	04:24	0:02	7	04:22	04:24	0:02
8	05:03	05:05	0:02	8	05:03	05:05	0:02	8	05:03	05:05	0:02	8	05:04	05:06	0:02
9	05:15	05:16	0:01	9	05:15	05:16	0:01	9	05:05	05:16	0:02	9	05:16	05:16	0:00
10	05:17	05:19	0:02	10	05:18	05:19	0:01	10	05:18	05:19	0:01	10	05:18	05:19	0:01
11	05:54	05:56	0:02	11	05:54	05:56	0:02	11	05:53	05:56	0:03	11	05:53	05:56	0:03
12	06:19	06:25	0:06	12	06:18	06:25	0:07	12	06:18	06:26	0:08	12	06:19	06:26	0:07
13	06:32	06:34	0:02	13	06:32	06:34	0:02	13	06:32	06:35	0:03	13	06:33	06:35	0:02
14	06:50	06:56	0:06	14	06:50	06:55	0:05	14	06:50	06:55	0:05	14	06:49	06:54	0:05
15	07:25	07:27	0:02	15	07:25	07:27	0:02	15	07:25	07:27	0:02	15	07:25	07:27	0:02
16	09:25	09:29	0:04	16	09:24	09:28	0:04	16	09:24	09:28	0:04	16	09:24	09:28	0:04
17	09:32	09:34	0:04	17	09:32	09:35	0:04	17	09:32	09:35	0:03	17	09:32	09:35	0:03
18	10:19	10:21	0:02	18	10:19	10:21	0:02	18	10:20	10:21	0:03	18	10:22	10:24	0:02
19	10:22	10:23	0:01	19	10:22	10:23	0:01	19	10:22	10:23	0:01	10	10.22	10.21	0.02
20	10:44	10:45	0:01		10.22	10.23	0.01	20	10:44	10:45	0:01	19	10:44	10:46	0:02
21	10:58	11:04	0:06	20	10:58	11:04	0:06	21	10:58	11:05	0:07	20	10:59	11:05	0:06
22	12:11	12:13	0:02	21	12:10	12:13	0:03	22	12:10	12:13	0:03	21	12:10	12:12	0:02
	12.11	12.15	0.02	22	13:08	13:08	0:00		12.10	12.15	0.03		12.10	12.12	0.02
23	13:21	13:22	0:01	23	13:21	13:22	0:01	23	13:21	13:22	0:01				
24	13:25	13:26	0:01	24	13:25	13:27	0:02	24	13:26	13:27	0:01	22	13:26	13:27	0:01
25	13:43	13:44	0:01	25	13:43	13:44	0:01	25	13:43	13:44	0:01		10.20	10.27	0.01
26	13:47	13:48	0:01	26	13:47	13:48	0:01	26	13:47	13:48	0:01	23	13:47	13:48	0:01
27	14:32	14:40	0:08	27	14:32	14:40	0:08	27	14:33	14:39	0:06	24	14:33	14:39	0:06
28	14:53	14:56	0:03	28	14:53	14:56	0:03	28	14:53	14:56	0:03	25	14:53	14:56	0:03
29	15:21	15:32	0:11	29	15:20	15:31	0:11	29	15:19	15:31	0:12	26	15:19	15:30	0:11
30	15:35	15:36	0:01	30	15:34	15:36	0:02	30	15:34	15:35	0:01	27	15:33	15:35	0:02
31	15:51	15:52	0:01	31	15:51	15:52	0:01	31	15:51	15:52	0:01	28	15:51	15:52	0:01
32	16:28	16:31	0:03	32	16:28	16:32	0:04	32	16:28	16:32	0:04	29	16:27	16:28	0:01
												30	16:29	16:32	0:03
33	16:50	16:57	0:07	33	16:49	16:57	0:08	33	16:49	16:57	0:08	31	16:50	16:58	0:08
34	17:45	17:48	0:03	34	17:45	17:48	0:03	34	17:45	17:48	0:03	32	17:45	17:48	0:03
35	18:47	18:53	0:06	35	18:45	18:53	0:08	35	18:45	18:52	0:07	33	18:45	18:51	0:06
36	19:11	19:12	0:01	36	19:11	19:12	0:01	36	19:11	19:12	0:01	34	19:10	19:12	0:02
37	19:37	19:38	0:01	37	19:36	19:38	0:02	37	19:36	19:38	0:02	35	19:36	19:38	0:02
38	19:43	19:49	0:06	38	19:42	19:49	0:07	38	19:41	19:48	0:07	36	19:40	19:48	0:08
39	21:22	21:24	0:02	39	21:21	21:24	0:03	39	21:21	21:23	0:02	37	21:21	21:23	0:02
40	21:56	21:58	0:02	40	21:56	21:58	0:02	40	21:55	21:58	0:03	38	21:55	21:58	0:03
41	22:16	22:17	0:01	41	22:15	22:17	0:02	41	22:15	22:17	0:02	39	22:15	22:17	0:02
42	22:39	22:44	0:05	42	22:38	22:44	0:06	42	22:38	22:44	0:06	40	22:40	22:44	0:04
43	23:07	23:08	0:01	43	23:07	23:08	0:01	43	23:06	23:08	0:02	41	23:06	23:07	0:01
			2:14				2:28				2:29				2:18

Thursday, March 3, 2005

1	Main St./	. & Mar Nash St	Č	Hi	ill St./W M	estern <i>I</i> ain St.	Ave. &			St. & Su Farboro		Ma		Thoma	as St. & St.
#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration
1	00:58	01:01	0:03	1	00:58	01:01	0:03	1	00:58	01:01	0:03	1	00:59	01:02	0:03
2	01:32	01:37	0:05	2	01:31	01:37	0:06	2	01:30	01:36	0:06	2	01:30	01:36	0:06
3	01:46	01:51	0:05	3	01:45	01:50	0:05	3	01:45	01:50	0:05	3	01:44	01:49	0:05
4	02:24	02:27	0:03	4	02:22	02:27	0:05	4	02:22	02:26	0:04	4	02:20	02:26	0:06
5	02:46	02:48	0:02	5	02:46	02:48	0:02	5	02:46	02:48	0:02	5	02:46	02:48	0:02
6	03:41	03:43	0:02	6	03:40	03:43	0:03	6	03:40	03:43	0:03	6	03:40	03:42	0:02
7	04:49	04:52	0:03	7	04:49	04:52	0:03	7	04:49	04:52	0:03	7	04:49	04:52	0:03
8	05:06	05:10	0:04	8	05:05	05:10	0:05	8	05:05	05:09	0:04	8	05:05	05:09	0:04
9	05:31	05:33	0:02	9	05:31	05:33	0:02	9	05:31	05:33	0:02	9	05:31	05:33	0:02
10	07:03	07:06	0:03	10	07:03	07:06	0:03	10	07:03	07:06	0:03	10	07:04	07:06	0:02
11	07:13	07:16	0:03	11	07:13	07:16	0:03	11	07:13	07:16	0:03	11	07:14	07:17	0:03
12	07:32	07:36	0:04	12	07:32	07:36	0:04	12	07:33	07:36	0:03	12	07:33	07:36	0:03
13	07:46	07:50	0:04	13	07:46	07:50	0:04	13	07:46	07:51	0:05	13	07:46	07:51	0:05
14	08:50	08:51	0:01	14	08:49	08:52	0:03	14	08:50	08:52	0:02	14	08:50	08:52	0:02
15	09:34	09:36	0:02	15	09:34	09:36	0:02	15	09:34	09:36	0:02	15	09:33	09:35	0:02
16	10:09	10:14	0:05	16	10:07	10:14	0:07	16	10:07	10:14	0:07	16	10:06	10:13	0:07
17	10:32	10:38	0:06	17	10:31	10:38	0:07	17	10:31	10:38	0:07	17	10:32	10:39	0:07
18	10:48	10:50	0:02	18	10:48	10:50	0:02	18	10:48	10:51	0:03	18	10:49	10:51	0:02
19	11:11	11:15	0:04	19	11:10	11:15	0:05	19	11:11	11:15	0:04	19	11:11	11:16	0:05
20	11:27	11:28	0:01	20	11:27	11:44	0:17	20	11:27	11:43	0:16	20	11:25	11:42	0:17
21	11:30	11:44	0:14									21	11.40	11.42	0.01
22	11:45	11:45	0:00	21	10.12	12.17	0.04	21	12.12	12.17	0.04	21	11:42	11:43	0:01
23	12:13 12:36	12:17 12:39	0:04	21	12:13 12:35	12:17 12:39	0:04	21	12:13 12:35	12:17 12:38	0:04	22	12:13 12:34	12:17 12:38	0:04
25	12:40	12:39	0:03	23	12:39	12:39	0:04	23	12:33	12:38	0:03	23	12:34	12:38	0:04
26	13:20	13:21	0:01	24	13:20	13:21	0:02	24	13:20	13:21	0:01	24	12.30	12.40	0.02
27	13:25	13:26	0:01	25	13:25	13:26	0:01	25	13:25	13:33	0:08	25	13:25	13:32	0:07
28	13:28	13:34	0:06	26	13:27	13:33	0:01	23	13.23	13.33	0.00	23	13.23	13.32	0.07
29	13:45	13:46	0:01	27	13:45	13:46	0:01	26	13:45	13:46	0:01				
30	13:48	13:49	0:01	28	13:48	13:50	0:02	27	13:49	13:50	0:01	26	13:49	13:50	0:01
31	13:54	13:55	0:01	29	13:54	13:55	0:01	28	13:54	13:55	0:01		10	10.00	0.01
32	13:56	13:57	0:01	30	13:56	13:57	0:01	29	13:56	13:57	0:01	27	13:56	13:58	0:02
33	14:53	14:54	0:01	31	14:53	14:54	0:01	30	14:53	14:54	0:01	28	14:52	14:54	0:02
34	15:23	15:23	0:00	32	15:22	15:23	0:01	31	15:22	15:23	0:01	29	15:22	15:23	0:01
35	15:44	15:47	0:03	33	15:44	15:47	0:03	32	15:44	15:48	0:04	30	15:45	15:48	0:03
36	16:19	16:21	0:02	34	16:19	16:27	0:08	33	16:19	16:21	0:02	31	16:19	16:21	0:02
37	16:22	16:27	0:05					34	16:21	16:27	0:06	32	16:22	16:28	0:06
38	16:52	16:53	0:01	35	16:52	16:53	0:01	35	16:52	16:53	0:01				
39	16:54	16:55	0:01	36	16:54	16:56	0:02	36	16:54	16:56	0:02	33	16:54	16:56	0:02
40	18:47	18:49	0:02	37	18:47	18:48	0:01	37	18:47	18:48	0:01	34	18:46	18:48	0:02
41	19:08	19:10	0:02	38	19:07	19:10	0:03	38	19:07	19:10	0:03	35	19:07	19:10	0:03
42	19:34	19:38	0:04	39	19:32	19:37	0:05	39	19:32	19:37	0:05	36	19:31	19:36	0:05
43	19:53	19:56	0:03	40	19:53	19:56	0:03	40	19:53	19:55	0:02	37	19:53	19:55	0:02
44	20:10	20:12	0:02	41	20:10	20:12	0:02	41	20:10	20:12	0:02	38	20:10	20:12	0:02
45	20:41	20:42	0:01	42	20:40	20:42	0:02	42	20:40	20:41	0:01	39	20:40	20:41	0:01
46	23:02	23:03	0:01	43	23:02	23:03	0:01	43	23:02	23:03	0:01	40	23:01	23:02	0:01
47	23:26	23:27	0:01	44	23:26	23:28	0:02	44	23:26	23:28	0:02	41	23:26	23:28	0:02
48	23:36	23:39	0:03	45	23:35	23:38	0:03	45	23:35	23:38	0:03	42	23:34	23:38	0:04
49	23:49	23:53	0:04	46	23:49	23:53	0:04	46	23:49	23:53	0:04	43	23:50	23:53	0:03
			2.14				2.26				2.20				2.20
			2:14	l			2:36				2:29	I			2:30

Friday, March 4, 2005

I	Main St./	. & Mar Nash St	_	H	ill St./W	estern A	Ave. &			St. & Su Farboro		Ma		z Thoma	as St. & St.
#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration
1	01:02	01:07	0:05	1	01:01	01:08	0:07	1	01:02	01:08	0:06	1	01:03	01:08	0:05
2	01:54	01:56	0:02	2	01:54	01:56	0:02	2	01:54	01:56	0:02	2	01:53	01:56	0:03
3	02:12	02:19	0:07	3	02:12	02:18	0:06	3	02:12	02:17	0:05	3	02:12	02:16	0:04
4	04:04	04:06	0:02	4	04:04	04:06	0:02	4	04:04	04:06	0:02	4	04:04	04:07	0:03
5	04:15	04:17	0:02	5	04:15	04:17	0:02	5	04:15	04:17	0:02	5	04:15	04:17	0:02
6	04:28	04:30	0:02	6	04:28	04:30	0:02	6	04:28	04:30	0:02	6	04:28	04:30	0:02
7	04:49	04:51	0:02	7	04:49	05:00	0:11	7	04:49	05:00	0:11	7	04:49	05:00	0:11
8	04:53	05:00	0:07	•		30.100	0.11	•	0 11 12	00100	0.12	•		30100	0.11
9	05:08	05:09	0:01	8	05:08	05:09	0:01	8	05:08	05:09	0:01	8	05:09	05:09	0:00
10	05:12	05:13	0:01	9	05:12	05:14	0:02	9	05:12	05:14	0:02	9	05:13	05:14	0:01
11	07:20	07:24	0:04	10	07:19	07:24	0:05	10	07:20	07:25	0:05	10	07:21	07:25	0:04
12	07:36	07:39	0:03	11	07:36	07:39	0:03	11	07:36	07:39	0:03	11	07:36	07:42	0:06
13	07:41	07:43	0:02	12	07:40	07:43	0:03	12	07:40	07:43	0:03				
14	08:02	08:03	0:01	13	08:02	08:03	0:01	13	08:02	08:03	0:01	12	08:02	08:03	0:01
15	09:40	09:46	0:06	14	09:38	09:45	0:07	14	09:38	09:45	0:07	13	09:36	09:45	0:09
16	10:18	10:23	0:05	15	10:18	10:23	0:05	15	10:18	10:23	0:05	14	10:19	10:23	0:04
17	11:47	11:49	0:02	16	11:46	11:49	0:03	16	11:46	11:48	0:02	15	11:46	11:48	0:02
18	12:01	12:03	0:02	17	12:01	12:03	0:02	17	12:02	12:03	0:01				
19	12:05	12:06	0:01	18	12:05	12:06	0:01	18	12:05	12:06	0:01	16	12:05	12:06	0:01
20	12:10	12:11	0:01	19	12:10	12:11	0:01	19	12:10	12:11	0:01	17	12:10	12:11	0:01
21	13:10	13:11	0:01	20	13:10	13:11	0:01	20	13:10	13:11	0:01				
22	13:14	13:15	0:01	21	13:14	13:15	0:01	21	13:14	13:15	0:01	18	13:14	13:15	0:01
23	13:34	13:35	0:01	22	13:33	13:35	0:02	22	13:33	13:35	0:02	19	13:34	13:35	0:01
24	13:41	13:42	0:01	23	13:41	13:42	0:01	23	13:41	13:42	0:01				
25	13:44	13:45	0:01	24	13:44	13:45	0:01	24	13:44	13:45	0:01	20	13:44	13:46	0:02
26	14:11	14:12	0:01	25	14:11	14:12	0:01	25	14:10	14:12	0:02	21	14:10	14:11	0:01
27	14:49	14:50	0:01	26	14:49	14:50	0:01	26	14:49	14:50	0:01	22	14:49	14:50	0:01
28	15:04	15:10	0:06	27	15:04	15:10	0:06	27	15:04	15:09	0:05	23	15:03	15:08	0:05
29	15:31	15:34	0:03	28	15:31	15:34	0:03	28	15:31	15:34	0:03	24	15:31	15:34	0:03
30	15:52	15:52	0:00	29	15:51	15:52	0:01	29	15:51	15:52	0:01	25	15:51	15:52	0:01
31	16:56	16:58	0:02	30	16:55	16:58	0:03	30	16:55	16:57	0:02	26	16:55	16:57	0:02
32	17:35	17:44	0:09	31	17:34	17:43	0:09	31	17:34	17:42	0:08	27	17:34	17:41	0:07
33	18:20	18:23	0:03	32	18:20	18:23	0:03	32	18:20	18:22	0:02	28	18:19	18:22	0:03
				33	18:28	18:28	0:00								
				34	18:29	18:29	0:00								
34	18:49	18:50	0:01	35	18:48	18:49	0:01	33	18:48	18:49	0:01	29	18:48	18:49	0:01
				36	18:51	18:52	0:01								
35	19:11	19:13	0:02	37	19:11	19:13	0:02	34	19:11	19:12	0:01	30	19:10	19:12	0:02
36	19:18	19:25	0:07	38	19:17	19:26	0:09	35	19:18	19:25	0:07	31	19:18	19:26	0:08
				39	19:37	19:38	0:01								
37	20:04	20:05	0:01	40	20:03	20:05	0:02	36	20:03	20:05	0:02	32	20:03	20:05	0:02
				41	20:05	20:06	0:01								
38	20:34	20:39	0:05	42	20:32	20:39	0:07	37	20:32	20:39	0:07	33	20:31	20:38	0:07
39	21:25	21:34	0:09	43	21:25	21:35	0:10	38	21:25	21:34	0:09	34	21:26	21:35	0:09
				44	21:35	21:37	0:02								
				45	21:47	21:47	0:00								
				46	21:48	21:48	0:00		_	_			_	_	
40	22:49	22:52	0:03	47	22:49	22:52	0:03	39	22:50	22:52	0:02	35	22:50	22:52	0:02
41	23:00	23:01	0:01	48	23:00	23:01	0:01	40	23:00	23:01	0:01	36	22:59	23:01	0:02
42	23:44	23:46	0:02	49	23:44	23:46	0:02	41	23:44	23:46	0:02	37	23:44	23:46	0:02
			1:59				2:21				2:04				2:01

Saturday, March 5, 2005

]	Main St./	. & Mar Nash St	_	Н	ill St./W	estern A	Ave. &			St. & Su Farboro		Ma		z Thoma	as St. & St.
#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration
$\frac{\pi}{1}$	00:41	00:49	0:08	π 1	00:40	00:49	0:09	π 1	00:40	00:48	0:08	1	00:38	00:48	0:10
2	01:02	01:03	0:01	2	01:02	01:03	0:01	2	01:02	01:04	0:03	2	01:02	01:04	0:02
3	01:02	01:30	0:04	3	01:02	01:30	0:04	3	01:02	01:04	0:04	3	01:02	01:04	0:02
4	02:06	02:08	0:04	4	02:06	02:08	0:04	4	02:06	02:08	0:04	4	02:05	02:08	0:04
5	03:42	02:08	0:05	5	03:40	03:46	0:02	5	03:40	02:08	0:02	5	03:39	02:08	0:03
6	04:20	04:23	0:03	6	04:20	04:23	0:03	6	04:20	04:23	0:03	6	03.39	04:22	0:03
7	04:27	04:29	0:03	7	04:27	04:29	0:03	7	04:27	04:29	0:03	7	04:19	04:29	0:03
8	05:08	05:12	0:02	8	05:08	05:12	0:02	8	05:08	05:12	0:02	8	05:09	05:13	0:04
9	05:49	05:50	0:04	9	05:49	05:50	0:04	9	05:49	05:50	0:04	0	03.09	03.13	0.04
10	05:51	05:52	0:01	10	05:51	05:52	0:01	10	05:51	05:53	0:02	9	05:51	05:53	0:02
11	06:04	06:10	0:06	11	06:03	06:10	0:07	11	06:03	06:11	0:08	10	06:05	06:19	0:02
12	07:06	07:09	0:03	12	07:06	07:09	0:03	12	07:06	07:09	0:03	11	07:06	07:08	0:02
13	09:47	09:52	0:05	13	09:46	09:52	0:06	13	07:00	07:09	0:06	12	09:46	07:08	0:05
14	10:06	10:07	0:03	14	10:05	10:08	0:03	14	10:06	10:08	0:00	13	10:06	10:08	0:03
15	10:41	10:43	0:02	15	10:03	10:43	0:03	15	10:41	10:43	0:02	14	10:42	10:44	0:02
16	11:39	11:44	0:05	16	11:37	11:43	0:02	16	11:37	11:43	0:02	15	11:36	11:42	0:06
17	11:56	12:00	0:03	17	11:56	12:00	0:04	17	11:56	12:01	0:05	16	11:57	12:01	0:04
18	12:09	12:25	0:16	18	12:07	12:24	0:04	18	12:06	12:23	0:03	17	12:05	12:21	0:04
10	12.09	12.23	0.10	10	12.07	12.24	0.17	10	12.00	12.23	0.17	18	12:22	12:22	0:00
19	12:50	12:54	0:04	19	12:48	12:53	0:05	19	12:47	12:53	0:06	19	12:46	12:53	0:07
20	12:59	13:00	0:04	20	12:59	13:00	0:03	20	12:59	13:00	0:00	19	12.40	12.33	0.07
21	13:03	13:04	0:01	21	13:03	13:00	0:01	21	13:03	13:00	0:01	20	13:03	13:05	0:02
22	13:26	13:04		22	13:26	13:04		22	13:03	13:03	0:02	20	15:05	15:05	0:02
23	13:20	13:27	0:01 0:01	23		13:27	0:01	23	13:27	13:28	0:01	21	12,21	12.22	0:01
24			0:01	23	13:31	13:32	0:01	23			0:01	22	13:31 14:12	13:32	0:01
25	14:13 14:19	14:13 14:20	0:00	25	14:12 14:19	14:13	0:01	25	14:12 14:19	14:14 14:20	0:02	22	14:12	14:13	0:01
26	14:19	14:20	0:01	26	14:19	14:20	0:01	26	14:19	14:20	0:01	23	14:23	14:24	0:01
27	15:13	15:14	0:02	27	15:13	15:14	0:01	27	15:13	15:14	0:01	24	15:13	15:14	0:01
28	16:13	16:18	0:01	28	16:12	16:18	0:01	28	16:12	16:19	0:01	25	16:13	16:19	0:01
29	17:13	17:17	0:03	29	17:12	17:17	0:05	29	17:12	17:16	0:04	26	17:12	17:16	0:04
30	18:06	18:13	0:04	30	18:05	18:13	0:03	30	18:06	18:13	0:04	27	18:07	18:14	0:04
31	18:36	18:37	0:07	31	18:35	18:37	0:08	31	18:35	18:37	0:07	28	18:35	18:37	0:07
32	18:46	18:47	0:01	32	18:46	18:47	0:02	32	18:46	18:47	0:02	29	18:46	18:47	0:02
33	18:55	18:57	0:02	33	18:55	18:58	0:03	33	18:55	18:58	0:03	30	18:56	18:58	0:02
34	19:27	19:29	0:02	34	19:26	19:29	0:03	34	19:26	19:28	0:03	31	19:26	19:28	0:02
35	19:41	19:44	0:02	35	19:41	19:44	0:03	35	19:41	19:44	0:02	32	19:40	19:44	0:02
36	19:41	19:44	0:03	36	19:41	19:44	0:03	36	19:41	19:44	0:03	33	19:40	19:44	0:04
37	20:06	20:08	0:02	37	20:06	20:08	0:02	37	20:06	20:08	0:01	34	20:06	20:08	0:02
38	21:22	21:26	0:02	38	20:00	21:25	0:02	38	21:21	21:25	0:02	35	21:21	20:08	0:02
39	22:07	22:08	0:04	39	22:07	22:09	0:04	39	22:07	22:09	0:04	36	22:07	22:09	0:03
40	22:40	22:41	0:01	40	22:40	22:41	0:02	40	22:39	22:40	0:02	37	22:39	22:40	0:02
41	23:02	23:07	0:01	41	23:01	23:07	0:01	41	23:01	23:07	0:01	38	23:02	23:07	0:01
42	23:49	23:51	0:03	42	23:49	23:51	0:08	41	23:49	23:51	0:08	39	23:49	23:51	0:03
42	23.49	23.31	0.02	+2	23.43	23.31	0.02	+4	23.49	23.31	0.02	27	23.49	23.31	0.02
			2:07				2:24				2:26				2:25
			=. 07	l			4.47	l			4.40	l			

Sunday, March 6, 2005

I	Main St./	. & Mar Nash St	_	Hi	ill St./W	estern A	Ave. &			St. & Su Farboro		Ma		Thoma	as St. & St.
#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration
1	01:35	01:38	0:03	1	01:34	01:38	0:04	1	01:34	01:38	0:04	1	01:33	01:38	0:05
2	01:56	01:57	0:03	2	01:56	01:57	0:04	2	01:56	01:57	0:04	2	01:56	01:57	0:03
3	02:04	02:07	0:03	3	02:04	02:07	0:03	3	02:04	02:07	0:03	3	02:05	02:07	0:02
4	02:48	02:51	0:03	4	02:48	02:51	0:03	4	02:48	02:51	0:03	4	02:49	02:52	0:02
5	02:55	03:03	0:03	5	02:54	03:02	0:08	5	02:54	03:02	0:08	5	02:53	03:01	0:08
6	03:20	03:23	0:03	6	03:20	03:02	0:03	6	03:20	03:02	0:03	6	03:20	03:01	0:03
7	03:39	03:41	0:03	7	03:39	03:41	0:03	7	03:39	03:41	0:03	7	03:40	03:42	0:02
8	05:09	05:41	0:02	8	05:09	05:11	0:02	8	05:09	05:11	0:02	8	05:09	05:42	0:02
9	05:35	05:39	0:04	9	05:35	05:39	0:04	9	05:35	05:39	0:04	9	05:36	05:39	0:03
10	05:44	05:45	0:01	10	05:44	05:45	0:01	10	05:44	05:45	0:01	10	05:45	05:45	0:00
11	05:47	05:49	0:02	11	05:48	05:49	0:01	11	05:48	05:49	0:01	11	05:48	05:49	0:01
12	07:23	07:30	0:02	12	07:22	07:30	0:01	12	07:22	07:30	0:08	12	07:21	07:29	0:01
13	07:36	07:38	0:07	13	07:36	07:38	0:02	13	07:36	07:38	0:02	13	07:36	07:38	0:02
14	08:05	08:09	0:04	14	08:05	08:10	0:05	14	08:05	08:10	0:05	14	08:06	08:10	0:04
15	08:27	08:32	0:05	15	08:26	08:31	0:05	15	08:26	08:31	0:05	15	08:26	08:30	0:04
16	08:47	08:49	0:03	16	08:46	08:49	0:03	16	08:47	08:49	0:03	16	08:47	08:49	0:04
17	09:06	09:09	0:02	17	09:06	09:09	0:03	17	09:06	09:09	0:02	17	09:05	09:08	0:02
18	09:50	09:52	0:03	18	09:48	09:52	0:03	18	09:48	09:51	0:03	18	09:47	09:51	0:03
19	10:11	10:13	0:02	19	10:11	10:13	0:04	19	10:11	10:13	0:03	19	10:11	10:14	0:03
20	10:28	10:19	0:02	20	10:28	10:19	0:02	20	10:28	10:13	0:02	20	10:11	10:14	0:00
21	10:32	10:33	0:01	21	10:33	10:34	0:01	21	10:33	10:34	0:01	21	10:33	10:34	0:01
22	10:53	10:59	0:06	22	10:52	10:59	0:07	22	10:53	10:59	0:06	22	10:54	10:59	0:05
23	13:20	13:21	0:00	23	13:20	13:21	0:01	23	13:21	13:21	0:00	22	10.54	10.57	0.03
24	13:24	13:25	0:01	24	13:24	13:25	0:01	24	13:24	13:25	0:00	23	13:24	13:26	0:02
25	13:35	13:36	0:01	25	13:35	13:36	0:01	25	13:35	13:36	0:01	23	13.24	13.20	0.02
26	13:37	13:38	0:01	26	13:37	13:38	0:01	26	13:38	13:39	0:01	24	13:38	13:39	0:01
27	14:17	14:27	0:10	27	14:17	14:27	0:10	27	14:18	14:27	0:09	25	14:18	14:28	0:10
	11117	11.27	0.10	28	14:37	14:38	0:01		11.10	11.27	0.07	20	11110	11.20	0.10
				29	14:59	15:00	0:01					26	14:59	15:00	0:01
28	15:23	15:24	0:01	30	15:23	15:24	0:01	28	15:22	15:24	0:02	27	15:22	15:23	0:01
29	15:38	15:43	0:05	31	15:36	15:43	0:07	29	15:36	15:43	0:07	28	15:35	15:42	0:07
30	15:53	15:57	0:04	32	15:53	15:56	0:03	30	15:52	15:56	0:04	29	15:52	15:55	0:03
31	17:12	17:19	0:07	33	17:12	17:20	0:08	31	17:12	17:20	0:08	30	17:13	17:20	0:07
32	17:33	17:35	0:02	34	17:32	17:35	0:03	32	17:32	17:35	0:03	31	17:32	17:35	0:03
33	17:47	17:51	0:04	35	17:46	17:54	0:08	33	17:46	17:54	0:08	32	17:47	17:54	0:07
34	17:51	17:54	0:03												
35	18:26	18:28	0:02	36	18:26	18:27	0:01	34	18:26	18:27	0:01	33	18:26	18:27	0:01
36	20:04	20:10	0:06	37	20:03	20:11	0:08	35	20:04	20:11	0:07	34	20:05	20:11	0:06
37	20:55	20:57	0:02	38	20:55	20:57	0:02	36	20:54	20:57	0:03	35	20:54	20:56	0:02
38	21:07	21:10	0:03	39	21:07	21:11	0:04	37	21:07	21:11	0:04	36	21:08	21:11	0:03
39	22:00	22:04	0:04	40	22:00	22:04	0:04	38	22:00	22:04	0:04	37	21:59	22:04	0:05
40	22:41	22:42	0:01	41	22:40	22:42	0:02	39	22:40	22:41	0:01	38	22:40	22:41	0:01
			2:05				2:20				2:14				2:06

Monday, March 7, 2005

]	Main St./	. & Mar Nash St	_	H	ill St./W	estern <i>I</i>	Ave. &			St. & Su Farboro		Ma		Thoma	as St. & St.
#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration	#	Started	Ended	Duration
1	00:24	00:26	0:02	1	00:24	00:26	0:02	1	00:24	00:26	0:02	1	00:23	00:25	0:02
2	01:21	01:28	0:07	2	01:19	01:27	0:08	2	01:19	01:27	0:08	2	01:18	01:26	0:08
3	02:32	02:39	0:07	3	02:30	02:38	0:08	3	02:29	02:36	0:07	3	02:28	02:35	0:07
4	03:02	03:04	0:02	4	03:01	03:04	0:03	4	03:01	03:04	0:03	4	03:02	03:04	0:02
5	03:10	03:16	0:06	5	03:09	03:14	0:05	5	03:08	03:14	0:06	5	03:08	03:13	0:05
6	03:57	04:00	0:03	6	03:57	04:00	0:03	6	03:57	04:01	0:04	6	03:57	04:01	0:04
7	04:12	04:14	0:02	7	04:12	04:14	0:02	7	04:12	04:14	0:02	7	04:12	04:14	0:02
8	04:24	04:35	0:11	8	04:23	04:34	0:11	8	04:23	04:34	0:11	8	04:23	04:34	0:11
9	05:45	05:47	0:02	9	05:45	05:47	0:02	9	05:46	05:47	0:01	9	05:46	05:47	0:01
10	05:50	05:52	0:02	10	05:51	05:52	0:01	10	05:51	05:52	0:01	10	05:51	05:52	0:01
11	06:58	07:00	0:02	11	06:58	07:00	0:02	11	06:58	07:00	0:02	11	06:58	07:01	0:03
12	07:23	07:26	0:03	12	07:23	07:26	0:03	12	07:23	07:26	0:03	12	07:24	07:27	0:03
13	09:49	09:51	0:02	13	09:49	09:51	0:02	13	09:49	09:51	0:02	13	09:50	09:51	0:01
14	10:52	10:52	0:00					14	10:47	10:49	0:02	14	10:47	10:49	0:02
15	12:06	12:07	0:01	14	12:06	12:07	0:01	15	12:06	12:07	0:01	15	12:06	12:08	0:02
16	12:42	12:49	0:07	15	12:41	12:48	0:07	16	12:41	12:48	0:07	16	12:40	12:47	0:07
17	13:11	13:12	0:01	16	13:11	13:12	0:01	17	13:11	13:12	0:01				
18	13:15	13:16	0:01	17	13:15	13:16	0:01	18	13:15	13:16	0:01	17	13:15	13:16	0:01
19	13:50	13:51	0:01	18	13:50	13:51	0:01	19	13:50	13:51	0:01				
20	13:53	13:54	0:01	19	13:53	13:55	0:02	20	13:54	13:55	0:01	18	13:54	13:55	0:01
				20	13:57	13:57	0:00								
21	14:23	14:23	0:00	21	14:22	14:23	0:01	21	14:22	14:23	0:01	19	14:22	14:23	0:01
22	14:42	14:43	0:01	22	14:42	14:43	0:01	22	14:42	14:43	0:01				
23	14:46	14:47	0:01	23	14:46	14:48	0:02	23	14:46	14:48	0:02	20	14:46	14:48	0:02
24	15:04	15:09	0:05	24	15:02	15:09	0:07	24	15:02	15:08	0:06	21	15:01	15:08	0:07
25	15:10	15:11	0:01	25	15:10	15:11	0:01	25	15:09	15:11	0:02	22	15:09	15:10	0:01
26	15:23	15:25	0:02	26	15:23	15:26	0:03	26	15:23	15:26	0:03	23	15:24	15:26	0:02
27	16:24	16:25	0:01	27	16:23	16:25	0:02	27	16:23	16:24	0:01	24	16:23	16:24	0:01
28	16:41	16:44	0:03	28	16:40	16:44	0:04	28	16:41	16:45	0:04	25	16:42	16:45	0:03
29	17:11	17:17	0:06	29	17:11	17:17	0:06	29	17:11	17:17	0:06	26	17:10	17:13	0:03
												27	17:14	17:17	0:03
30	17:41	17:42	0:01	30	17:40	17:42	0:02	30	17:40	17:42	0:02	28	17:40	17:42	0:02
31	18:39	18:47	0:08	31	18:37	18:46	0:09	31	18:37	18:45	0:08	29	18:36	18:44	0:08
32	19:14	19:15	0:01	32	19:14	19:15	0:01	32	19:14	19:15	0:01	30	19:13	19:15	0:02
33	19:55	19:57	0:02	33	19:55	19:57	0:02	33	19:55	19:56	0:01	31	19:54	19:56	0:02
34	20:05	20:08	0:03	34	20:03	20:08	0:05	34	20:03	20:08	0:05	32	20:03	20:07	0:04
35	21:37	21:38	0:01	35	21:37	21:38	0:01	35	21:37	21:39	0:02	33	21:37	21:39	0:02
36	23:16	23:19	0:03	36	23:15	23:19	0:04	36	23:16	23:18	0:02	34	23:16	23:18	0:02
			1:42				1:56				1:53				1:48
			1:42				1:50				1:55				1:48

APPENDIX B:

EXISTING (2003) TURNING MOVEMENT COUNTS

AND

AVERAGE ANNUAL DAILY TRAFFIC

Rocky Mount Downtown Circulation Study Historical Average Daily Traffic Volumes

Descript_1	COUNT_YR1	AADT_YR1	COUNT_YR2	AADT_YR2	COUNT_YR3	AADT_YR3	COUNT_YR4	AADT_YR4	NAME_OF_RO	2000_VOLUM	2001_VOLUM	2025_VOLUM
Between Falls Rd & Aven St. on Grand Ave.	1999	9500	1997	8100	1996	9300	1995	8100		0	9400	0
Between Albemarle Ave. & Main ST. on Grand Ave.	1999	11000	1997	11000	1996	12000	1995	12000		0	11000	0
Between E Grand Ave. & E Highland Ave. on Atlanti	1999	5300	1997	5000	1996	6700	1995	6700		0	5200	0
Church St. above Goldleaf St.	1999	5100	1997	5000	1996	4900	1995	5400		0	4200	0
Between Thomas St. & Gay St. on Franklin St.	1999	4400	1997	3500	1996	4000	1995	4300		0	0	0
Between Pearl St. & Franklin St. on Thomas St.	1999	5500	1997	7500	1996	8000	1995	7700		0	4900	0
Between Church St. & Main St. on Thomas St.	1999	4000	1997	4400	1996	4600	1995	5100		0	3500	0
Between Western Ave. & Sunset Ave. on Franklin St.	1999	4700	1997	4100	1996	4500	1995	4800		0	4500	0
Between Main St. & Church St. on Tarboro St.	1999	4500	1997	4800	1996	6000	1995	6000		0	4400	0
Between Western Ave. & Sunset Ave. on Church St.	1999	5800	1997	5600	1996	7500	1995	7500		0	5900	0
Between Arlington St. & Washington St. on Hill St.	1999	2600	1997	4200	1996	4100	1995	5200		0	2400	0
Between Eastern Ave. & Tarboro St. on Atlantic Ave	1999	7600	1997	7200	1996	6800	1995	8000		0	6400	0
Arlington St. @ Raleigh St.	1999	4400	1997	4500	1996	4600	1995	4600		0	4900	0
Below Rocky St. on Franklin St.	1999	3300	1997	3100	1996	4300	1995	4300		0	2700	0
Nash County Railroad	1997	3200	1996	4000	1995	4100	1994	4000	Church St.	3400	3500	5600
Between Arlington St. & Washington St. on George S	1999	12000	1997	12000	1996	12000	1995	12000		0	1000	0
Below George St. on Franklin St.	1999	3600	1997	3900	1996	5000	1995	5200		0	3100	0
Between Grace St. & Pearl St. on George St.	1999	10000	1997	11000	0	0	0	0		0	1000	0
Between Albemarle Ave. & Arlington St. on Tarboro	1999	3800	1997	3400	0	0	0	0		0	4300	0
Below Nash St. on Franklin St.	1999	4200	1997	3800	0	0	0	0		0	3800	0
Gay St. @ Grace St.	1999	11000	1997	10000	0	0	0	0		0	1100	0
Between Main St. & Church St. on E Grand Ave.	1996	12000	1994	11000	1993	12000	1983	12000		0	1100	0
Nash County Railroad	0	0	0	0	0	0	0	0	Franklin St.	3300	0	5500
Nash County Railroad	0	0	0	0	0	0	0	0	Pearl St.	500	0	800
Nash County Railroad	0	0	0	0	0	0	0	0	Grace St.	6900	0	11300
CSX "A" LINE	0	0	0	0	0	0	0	0	E. Grand Ave.	11800	0	19400
CSX "A" LINE	0	0	0	0	0	0	0	0	Goldleaf St.	2500	0	3700
CSX "A" LINE	0	0	0	0	0	0	0	0	Thomas St.	4700	0	7800
CSX "A" LINE	0	0	0	0	0	0	0	0	Sunset Ave.	5100	0	8400
CSX "A" LINE	0	0	0	0	0	0	0	0	Western Ave.	4400	0	6400
CSX "A" LINE	0	0	0	0	0	0	0	0	Nash St.	2300	0	3400

Groups Printed- Unshifted

City of Rocky Mount
Engineering Dept.
One Government Plaza File Name: Main St. & Marigold St.-Nash S

Rocky Mount, NC 27804 Site Code : 01421108 Start Date : 7/22/2003

		01.	1 A P = 1	. Ct						Groups	FILL		Main				k.A.	arigold	St				
			V Mair				141	estbou	ınd				: Main orthbol					astbou					
	1	30	uthbo	una							—	I									Excl	Inclu.	
Start	Left	Thr	Rig	Pe	App.	Left	Thr	Rig	Pe	App.	Left	Thr	Rig	Pe	App.	Left	Thr	Rig	Pe	App.	u.	Tota	Int.
Time	LUIT	U	ht	ds	Total		u	ht	ds	Total	-0	u	ht	ds	Total		u	ht	ds	Total	Total	ŧ	Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0				
07:00 AM	2	4	0	1	6	0	0	0	1	0	0	1	1	2	2	0	5	1	0	6	4	14	18
07:15 AM	ō	2	ō	Ó	2	Ö	Ō	Ō	2	Ó	0	0	2	1	2	3	12	1	0	16	3	20	23
07:30 AM	1	8	ō	ō	9	0	Ó	Ō	0	0	0	0	1	4	1	1	20	0	0	21	4	31	35
07:45 AM	3	10	Ō	1	13	0	0	0	0	0	0	1	2	0	3	2	28	3	0	33	1	49	50
Total	6	24	0	2	30	0	0	0	3	0	0	2	6	7	8	6	65	5	0	76	12	114	126
08:00 AM	5	10	0	0	15	0	0	0	0	0	0	3	1	0	4	2	20	7	0	29	0	48	48
08:15 AM	2	15	0	0	17	0	0	0	0	0	0	3	0	1	3	2	15	0	0	17	1	37	38
08:30 AM	2	8	0	0	10	0	0	0	0	0	0	2	0	0	2	2	16	1	0	19	0	31	31
08:45 AM	2	14	0	0	16	0	0	0	0	0	0	3	4	0	7	2	22	1_	0	25	0	48	48
Total	11	47	0	0	58	0	0	0	0	0	0	11	5	1	16	8	73	9	0	90	1	164	165
11.00 AM	5	12	0	1	17	0	0	0	0	0	0	2	0	5	2	1	20	3	0	24	6	43	49
11:00 AM		19	0	2	29	0	Ö	0	0	0	0	2	3	2	5	4	13	2	ŏ	19	4	53	57
11:15 AM	10 16	21	0	0	37	Ö	0	ő	0	Ö	ő	1	2	1	3	0	16	4	ŏ	20	1	60	61
11:30 AM	8	28	Ö	0	36	ő	0	Ö	0	0	ő	3	3	2	6	3	10	5	2	18	4	60	64
11:45 AM Total	39	80	0	3	119	0	0	ŏ	0	- 0	0	8	8	10	16	8	59	14	- 2	81	15	216	231
Total	35	OU	U	,	113	, ,	٠	v	v	· ·	, 0	٠	•	10	,0			• • •	-	0.	, ,,	2.0	
12:00 PM	12	17	0	0	29	0	0	0	0	0	0	3	1	0	4	3	22	0	0	25	0	58	58
12:15 PM	9	15	Ō	0	24	0	0	0	0	0	0	2	3	1	5	6	26	0	0	32	1	61	62
12:30 PM	9	15	Ō	Ó	24	0	0	0	0	0	o	2	2	3	4	1	22	1	0	24	3	52	55
12:45 PM	12	12	Ō	1	24	0	0	0	1	0	0	2	1	0	3	8	19	3	0	30	2	57	59
Total	42	59	Ō	1	101	0	0	0	1	0	0	9	7	4	16	18	89	4	0	111	6	228	234
	_	_	_	_	_		_	_	_	_				_	•	١ ،	_	_	^				_
03:45 PM	0_	0	ő	0	0	0	. 0	0	0	0	0	2	1	0	3	0	0	0	0	0	0	3	3
Total	0	0	Ö	0	0	0	0	0	0	0	0	2	1	0	3	U	U	U	U	U	U	3	3
04:00 PM	5	18	0	0	23	0	0	0	0	0	1 o	0	2	0	2	6	29	1	0	36	0	61	61
04:00 PM	14	14	Ö	Ö	28	Ö	ő	Ö	ŏ	0	ŏ	ŏ	2	2	2	4	24	Ó	ő	28	2	58	60
04:30 PM	11	10	0	Ö	21	Ö	Ö	Ö	ő	0	ő	Ö	1	ō	1	2	29	5	1	36	1	58	59
04:30 FM	8	10	0	Ö	18	ő	Ö	ő	ŏ	0	Ö	2	3	Õ	5	1	32	3	Ö	36	Ö	59	59
Total	38	52	ŏ		90	ő	ő	- 0	- 0	0	0	2	8	- 2	10	13	114	9	- 1	136	3	236	239
10121	30	V.	•			•	•	•	•	•	, •	_	•	_				-	•		,		
05:00 PM	10	17	0	0	27	0	0	0	0	0	0	1	0	0	1	0	32	0	0	32	0	60	60
05:15 PM	7	6	0	0	13	0	0	0	0	0	0	2	1	0	3	0	25	0	0	25	0	41	41
05:30 PM	1	7	0	1	8	0	0	0	0	0	0	1	0	0	1	1	12	0	0	13	1	22	23
05:45 PM	4	8	0	0	12	0	0	0	0	0	0	1	3	0	4	1	28	1	0	30	0	46	46
Total	22	38	0	1	60	0	0	0	0	0	0	5	4	0	9	2	97	1	0	100	1	169	170
Grand	158	300	0	7	458	0	0	0	4	0	0	39	39	24	78	55	497	42	3	594	38	1130	1168
Total			Ŭ	Г	700		•	•	7	3	"				. 5			1 =	•				
Apprch %	34.	65.	0.0			0.0	0.0	0.0			0.0	50.	50.			9.3	83.	7.1					
	5	5										0	0				7						
Total %	14.	26.	0.0		40.5	0.0	0.0	0.0		0.0	0.0	3.5	3.5		6.9	4.9	44. 0	3.7		52.6	3.3	96.7	

City of Rocky Mount Engineering Dept.

One Government Plaza File Name: Main St. & Marigold St.-Nash S

Rocky Mount, NC 27804 Site Code : 01421108

Start Date : 7/22/2003

-			fain St.			Wes	tbound				Main St. hbound				gold St. bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
'eak Hour Fron			8:45 AM	- Peak 1	of 1						,	-	1				
Intersection			•	£4		^		^		7	4	4.4	7	83	10	100	165
Volume	11 20.4	43 79.6	0.0	54	0.0	0.0	0 0.0	0	0.0	7 63.6	4 36.4	11	7.0	83.0	10.0	100	100
Percent 07:45									1				l				
Volume	3	10	0	13	0	0	0	0	0	1	2	3	2	28	3	33	49
Peak Factor									1								0.842
High Int.	08:15 A	М			6:45:00	MA (08:00	۹М			07:45 A				
Volume	2	15	0	17	0	0	0	0	0	3	1	4	2	28	3	33	
Peak Factor				0.794	,				İ			0.688				0.758	
eak Hour Fron			8:45 AM	- Peak 1					100.00				1 07.00 4				
By Approach					07:00		•	^	08:00		_	16	07:30 A 7		10	100	
Volume	11 19.0	47 81.0	0 0.0	58	0	0	0	0	0.0	11 68.8	5 31.3	10	7.0	83 83.0	10.0	100	
Percent High Int.			0.0			-	-		08:45		31.3		07:45 A		10.0		
Volume	2	15	0	17	_	-	_	_	00.437	3	4	7	2	28	3	33	
Peak Factor	_		ŭ	0.853				-		·		0.571	-		_	0.758	
eak Hour Fror			2:45 PM	- Peak 1	of 1				1				1			1	
Volume	45	NVI 81	0	126	0	0	0	0	٥	9	9	18	12	74	9	95	239
Percent	35.7	64.3	0.0	120	0.0	0.0	0.0	v	0.0	50.0	50.0	10	12.6	77.9	9.5	"	200
12:15				0.4					1			_				22	04
Volume	9	15	0	24	0	0	0	0	0	2	3	5	6	26	0	32	61
Peak Factor																	0.980
High Int.			_		_		_	_	11:45 /		_	_	12:15 F		_		
Volume Peak Factor	16	21	0	37 0.851	0	0	0	0	0	3	3	6 0.750	6	26	0	32 0.742	
	. 44.00		0.4C DI4		-4				'				•			•	
'eak Hour Fron By Approach			2:45 PIVI	- Peak I	11:00 A	\ N.4			11:45	A R A			12:00 F	DKA .		1	
Volume	46	85	0	131	0	0	0	0	0	10	9	19	18	89	4	111	
Percent	35.1	64.9	0.0	,,,,	_	-	-	·	0.0	52.6	47.4		16.2	80.2	3.6		
High Int.	11:30 A		*		_				11:45 /				12:15 F				
Volume	16	21	0	37	-	-	-	-	0	3	3	6	6	26	0	32	
Peak Factor				0.885				-				0.792				0.867	
eak Hour Fron			5:45 PM	- Peak 1	of 1											•	
Intersection			_		_	_	_	_		_	_					400	
Volume	38	52	0	90	0	0	0	0	0.0	2	8	10	13 9.6	114	9 6.6	136	236
Percent	42.2	57.8	0.0		0.0	0.0	0.0			20.0	80.0			83.8			
04:00 Volume	5	18	0	23	0	0	0	0	0	0	2	2	6	29	1	36	61
Peak Factor																	0.967
High Int.	04:15 P	М							04:45 F	PM			04:00 F	M			
Volume	14	14	0	28	0	0	0	0	0	2	3	5	6	29	1	36	
Peak Factor				0.804	ļ							0.500				0.944	
eak Hour Fron			5:45 PM	- Peak 1													
By Approach			_		04:00 F		_	_	04:00 F		_	4.5	04:00 F		^	400	
Volume	43	51 54.2	0	94	0	0	0	0	0	20.0	8	10	13	114 83.8	9 6.6	136	
Percent High Int.	45.7	54.3	0.0			-	-		0.0 04:45 F	20.0 ⊃M	80.0		9.6 04:00 F		0.0		
Volume	14	14	0	28	ļ .	_	-	_	04.43 F	-wi 2	3	5	6	29	1	36	
Peak Factor	ידיו	1-7	J	0.839		-	_	-		_	Ü	0.500			•	0.944	
				0.000	•				1				1			, , 1	

File Name $\,:$ Church St. & Falls Rc

Site Code : 01420110 Start Date : 5/21/2003

		0 "					s Rd.	Printed-		Chu	rch St. nbound				ls Rd. tbound		
		South	nbound	App.		wes	tbound	App.				App.				App.	In
Start Time	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Tota
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	0 1	0	0	0	0	8	2	10	13	33	1	47	1	7	0	8	6
07:15 AM	Ö	0	0	0	0	9	1	10	10	34	1	45	0	8	0	8	6
07:30 AM	0	0	0	0	0	15	5	20	15	40	0	55	0	12	0	12	8
07:45 AM	0	0	0	0	0	15	7	22	22	37	3	62	0	10	0	10	9
Total	0	0	0	0	0	47	15	62	60	144	5	209	1	37	0	38	30
08:00 AM	0	0	0	0	0	7	7	14	16	53	0	69	0	11	0	11	ę
08:15 AM	0	0	0	0	0	9	4	13	20	37	3	60	0	12	0	12	8
08:30 AM	0	0	0	0	0	7	5	12	32	31	1	64	1	11	0	12	
08:45 AM	0	0	0	0	0	4	2	6	21	37	0	58	0	0	0	0	
Total	0	0	0	0	0	27	18	45	89	158	4	251	1	34	0	35	3
11:00 AM	0	0	0	0	0	7	0	7	23	39	0	62	0	5	0	5	
11:15 AM	ŏ	ŏ	Õ	Ö	ŏ	17	2	19	24	47	Õ	71	1	5	0	6	
11:30 AM	ŏ	ŏ	ő	0	ŏ	7	2	9	42	55	ō	97	2	12	0	14	1
11:45 AM	ŏ	ŏ	Õ	Ö	Õ	19	1	20	40	39	Ō	79	2	11	0	13	1
Total	0	0	0	Ö	0	50	5	55	129	180	0	309	5	33	0	38	4
12:00 PM	0	0	0	0	0	20	2	22	50	45	0	95	1	3	0	4	1
12:15 PM	0	0	0	0	0	8	3	11	50	40	0	90	0	4	0	4	1
12:30 PM	0	0	0	0	0	7	4	11	23	41	2	66	3	11	0	14	
12:45 PM	0	0	0	0	0	12	2	14	48	45	0	93	3	16	0	19	1
Total	0	0	0	0	0	47	11	58	171	171	2	344	7	34	0	41	4
04:00 PM	0	0	0	0	0	19	5	24	24	55	5	84	0	10	0	10	1
04:15 PM	0	0	0	0	0	20	5	25	42	59	0	101	1	19	0	20	1
04:30 PM	0	0	0	0	0	15	4	19	44	41	2	87	3	18	0	21	1
04:45 PM	0	0	0	0	0	18	1_	19	41	58	0	99	0	11	0	11	
Total	0	0	0	0	0	72	15	87	151	213	7	371	4	58	0	62	5
05:00 PM	0	0	0	0	0	16	6	22	65	98	3	166	1	4	0	5	1
05:15 PM	0	0	0	0	0	23	6	29	46	56	0	102	1	19	0	20	1
05:30 PM	0	0	0	0	0	12	0	12	42	48	2	92	1	12	0	13	1
05:45 PM	0	0	0	0	0_	7	0	7	33	39	0	72	<u>0</u>	13 48	0	13	5
Total	0	0	0	0	0	58	12	70	186	241	5	432	3	48	U	51	ŧ
rand Total	0	0	0	0	0	301	76	377	786	1107	23	1916	21	244	0	265	25
Apprch %	0.0	0.0	0.0		0.0	79.8	20.2		41.0	57.8	1.2	74.0	7.9	92.1	0.0	10.4	
Total %	0.0	0.0	0.0	0.0	0.0	11.8	3.0	14.7	30.7	43.3	0.9	74.9	0.8	9.5	0.0	10.4	

File Name: Church St. & Falls Ro

Site Code : 01420110 Start Date : 5/21/2003

· -		South	hbound	-			s Rd. tbound				rch St.				s Rd. bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
eak Hour Fron	n 07:00	AM to 0	8:45 AM	- Peak 1	of 1		•										
Intersection,	_07:45 A	(M)													_		
Volume	0	0	0	0	0	38	23	61	90	158	7	255	1	44	0	45	361
Percent	0.0	0.0	0.0		0.0	62.3	37.7		35.3	62.0	2.7		2.2	97.8	0.0		
08:00	0	0	0	0	0	7	7	14	16	53	0	69	0	11	0	11	94
Volume	U	U	•	·		•	•		'0	-	v		•		•		7 1
Peak Factor					Ì												0.960
High Int.		AM			07:45 A				08:00 A				08:15 A				
Volume	0	0	0	0	0	15	7	22	16	53	0	69	0	12	0	12	
Peak Factor								0.693				0.924				0.938	
'eak Hour Fron			8:45 AM	- Peak 1													
By Approach					07:30 A				07:45 A				07:30 A				
Volume	0	0	0	0	0	46	23	69	90	158	7	255	0	45	0	45	
Percent	-	-	-		0.0	66.7	33.3		35.3	62.0	2.7		0.0	100.0	0.0		
High Int.	-				07:45 A	M			08:00 A				07:30 A				
Volume	-	-	-	-	0	15	7	22	16	53	0	69	0	12	0	12	
Peak Factor				-				0.784				0.924				0.938	
eak Hour Fron			2:45 PM	- Peak 1	of 1				1				ı				
Intersection				_	_		_				_		_		_		
Volume	0	0	0	0	0	54	8	62	182	179	0	361	5	30	0	35	458
Percent	0.0	0.0	0.0		0.0	87.1	12.9		50.4	49.6	0.0		14.3	85.7	0.0		
12:00	0	0	0	0	0	20	2	22	50	45	0	95	1	3	0	4	121
Volume	·	•	U	·	Ů	LU	-		00	70	·		•	ŭ	•	'	
Peak Factor																	0.946
High Int.					12:00 F				11:30 A				11:30 A				
Volume	0	0	0	0	0	20	2	22	42	55	0	97	2	12	0	14	
Peak Factor					l			0.705				0.930				0.625	
eak Hour Fron			2:45 P M	- Peak 1													
By Approach	11:00 A	M			11: 1 5 A				11:30 A				12:00 F				
Volume	0	0	0	0	0	63	7	70	182	179	0	361	7	34	0	41	
Percent	-	-	-		0.0	90.0	10.0		50.4	49.6	0.0		17.1	82.9	0.0		
High Int.	-				12:00 F	M			11:30 A	\M			12:45 F	M			
Volume	-	-	-	-	0	20	2	22	42	55	0	97	3	16	0	19	
Peak Factor				-				0.795				0.930				0.539	
eak Hour Fron			5:45 PM	- Peak 1	of 1												
Intersection		M 👌															
Volume	N	. 0	0	0	0	72	17	89	196	253	5	454	5	52	0	57	600
Percent	0.0	0.0	0.0		0.0	80.9	19.1		43.2	55.7	1.1		8.8	91.2	0.0		
05:00	0	0	0	0	0	16	6	22	65	98	3	166	1	4	0	5	193
Volume	v	V	U	U		10	U	44	"	90	J	100	'	7	U	١,	
Peak Factor																İ	0.777
High Int,					05:15 F				05:00 F				04:30 F			İ	
Volume	0	0	0	0	0	23	6	29	65	98	3	166	3	18	0	21	
Peak Factor								0.767				0.684				0.679	
eak Hour Fron	n 04:00 I	PM to 0	5:45 PM	- Peak 1													
By Approach					04:30 P	M			04:45 F	M		i	04:00 F	M			
Volume	0	0	0	0	0	72	17	89	194	260	5	459	4	58	0	62	
					0.0	80.9	19.1		42.3	56.6	1.1		6.5	93.5	0.0		
Percent	-	-	•		0.0	00.5	10.1		42.0	JQ.U					0.0		
	-	-	-		05:15 P		13.1		05:00 F		•••		04:30 F		0.0		
Percent		-	-	-			6	29 0.767	05:00 F 65		3	166 0.691			0.0	21 0.738	

City of Rocky Mount
Engineering Dept.
One Government PlaizaName: Cokey Rd. & George St. & Tarboro St
Rocky Mount, NC 278824Code: 01421145
Start Date: 7/2/2003

Page No : 1

Groups Printed- 1 - Unshifted

			rge St.				oro St. tbound	100 1 11110			rge St.				arboro astbou			
Start Time	Near Left	Far Left	Thru	App. Total	Left	Thru	Right	App. Total	Thru	Far Right	Near Righ	App. Total	Left	Thru -Tar	Thru -Cok	Right	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	boro 1.0	ey 1.0	1.0	-	
07:00 AM	2	3	25	30	3	- 1.0	0	3	18	5	2	25	7	9	14	0	30	88
07:15 AM	Ó	8	24	32	8	ŏ	4	12	54	3	1	58	7	7	21	2	37	139
07:30 AM	Ö	10	44	54	11	ŏ	7	18	45	8	1	54	3	ò	12	2	17	143
07:45 AM	Ö.	4	28	32	7	ŏ	4	11	53	4	1	58	8	5	32	2	47	148
Total	2	25	121	148	29	Ö	15	44	170	20	5	195	25	21	79	6	131	518
08:00 AM	2	5	31	38	21	0	5	26	53	7	4	64	4	6	14	6	30	158
08:15 AM	3	11	38	52	2	0	0	2	11	1	1	13	6	3	12	1	22	89
08:30 AM	1	4	40	45	23	0	16	39	74	17	3	94	13	10	19	2	44	222
08:45 AM	1	9	42	52	18	0	6	24	69	11	2	82	9	8	12	8	37	195
Total	7	29	151	187	64	0	27	91	207	36	10	253	32	27	57	17	133	664
11:00 AM	10	7	49	66	8	0	11	19	46	9	0	55	17	29	26	6	78	218
11:15 AM	3	8	37	48	13	Ō	10	23	58	12	1	71	19	10	13	5	47	189
11:30 AM	3	7	31	41	14	ō	4	18	37	8	1	46	9	9	10	4	32	137
11:45 AM	Ö	8	46	54	9	0	11	20	70	11	1	82	23	17	15	3	58	214
Total	16	30	163	209	44	0	36	80	211	40	3	254	68	65	64	18	215	758
12:00 PM	0	11	40	51	15	0	16	31	54	9	4	67	18	11	17	7	53	202
12:15 PM	0	5	38	43	10	0	8	18	73	9	4	86	30	13	38	4	85	232
12:30 PM	0	5	39	44	11	0	10	21	51	10	1	62	18	14	13	6	51	178
12:45 PM	2	6	29	37	14	0	13	27	59	16	4	79	9	13	25	2	49	192
Total	2	27	146	175	50	0	47	97	237	44	13	294	75	51	93	19	238	804
04:00 PM	0	9	27	36	7	0	9	16	143	14	4	161	34	23	30	7	94	307
04:15 PM	2.	12	59	73	16	0	15	31	169	22	5	196	19	20	23	2	64	364
04:30 PM	0	9	84	93	21	0	7	28	209	14	0	223	24	18	39	12	93	437
04:45 PM	0	11	40	51	34	0	33	67	175	9	1	185	18	22	22	3_	65	368
Total	2	41	210	253	78	0	64	142	696	59	10	765	95	83	114	24	316	1476
05:00 PM	0	5	44	49	13	0	11	24	94	8	0	102	45	32	37	8	122	297
05:15 PM	1	9	44	54	34	0	25	59	103	11	5	119	47	17	49	9	122	354
05:30 PM	1	8	40	49	18	0	10	28	77	16	2	95	19	24	21	4	68	240
05:45 PM Total	<u>6</u> 8	28 50	82 210	116 268	39 104	0	<u>5</u> 51	44 155	86 360	13 48	9	101 417	37 148	27 100	40 147	7 28	423	372 1263
Grand Total	37	202	1001	1240	369	0	240	609	1881	247	50	2178	443	347	554	112	1456	5483
Approh %	3.0	16.3	80.7	12.75	60.6	0.0	39.4	000	86.4	11.3	2.3		30.4	23.8	38.0	7.7		
Total %	0.7	3.7	18.3	22.6	6.7	0.0	4.4	11.1	34.3	4.5	0.9	39.7	8.1	6.3	10.1	2.0	26.6	
1 Juli 70	0.1	J.,	. 0.0	-2.0	٥.,	3.0	TT		01.0	1.0	5.0		٠.٠	3.0				

City of Rocky Mount
Engineering Dept.
One Government PlaiteName: Cokey Rd. & George St. & Tarboro St
Rocky Mount, NC 27804Code: 01421145
Start Date: 7/2/2003

Page 1	νo	:	2
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			rge St.	·			oro St. tbound				rge St.				arboro astbou			
Died Ties	Near	Far		App.	Loft			Арр.	Thru	Far	Near	App.	Left	Thru -Tar	Thru -Cok	Right	App.	Int.
Start Time	Left	Left	Thru	Total	Left	Thru	Right	Total	IIIIu	Right	Righ t	Total	Leit	boro	ey	Night	Total	Total
Peak Hour Fro	m 07:00	AM to	09:45	AM - Peal	k 1 of 1	<u> </u>	l			1						1		
Intersection						_			007		40	050		07	c-7	47	422	664
Volume	7	29	151	187	64	0	27	91	207 81.8	36 14.2	10	253	32 24.1	27 20.3	57 42.9	17 12.8	133	664
Percent 08:30	3.7	15.5	80.7		70.3	0.0	29.7				4.0							
Volume	1	4	40	45	23	0	16	39	74	17	3	94	13	10	19	2	44	222
Peak Factor																	ļ	0.748
High Int.	08:15	AM			08:30				08:30		_		08:30			_		
Volume	3	11	38	52	23	0	16	39	74	17	3	94	13	10	19	2	44	
Peak Factor				0.899	l			0.583				0.673	ŀ				0.756	
Peak Hour Fro	m 07:00	AM to	09:45	AM - Peal	k 1 of 1				1				ı				1	
By	08:00	АМ			08:00	AM			08:00	AM			07:45	AM				
Approach Volume	7	29	151	187	64	0	27	91	207	36	10	253	31	24	77	11	143	
Percent	3.7	15.5	80.7	107	70.3	0.0	29.7	٥.	81.8	14.2	4.0	200	21.7	16.8	53.8	7.7		
High Int.					08:30				08:30	AM			07:45					
Volume	3	11	38	52	23	0	16	39	74	17	3	94	8	5	32	2	47	
Peak Factor				0.899				0.583				0.673					0.761	
Peak Hour Fro	m 10:00	AM to	01:45	PM - Peal	k 1 of 1													
Intersection													1					
Volume	0	29	163	192	45	0	45	90	248	39	10	297	89	55	83	20	247	826
Percent	0.0	15.1	84.9		50.0	0.0	50.0		83.5	13.1	3.4		36.0	22.3	33.6	8.1		
12:15	0	5	38	43	10	0	8	18	73	9	4	86	30	13	38	4	85	232
Volume Peak Factor																		0.890
High Int.	11:45	AM		!	12:00	РМ			12:15	PM			12:15	PM				
Volume	0	8	46	54	15	0	16	31	73	9	4	86	30	13	38	4	85	
Peak Factor				0.889				0.726				0.863	1				0.726	
Peak Hour Fro	m 10:00	AM to	01:45	PM - Peal	k 1 of 1													
Ву	11:00	АМ			12:00	РМ			11:45	AM			11:45	AM				
Approach			400		İ		47	0.7			40	207	89		83	20	247	
Volume Percent	16 7.7	30 14.4	163 78.0	209	50 51.5	0.0	47 48.5	97	248 83.5	39 13.1	10 3.4	297	36.0	55 22.3	33.6	8.1	241	
High Int.	11:00		70.0		12:00		40.0		12:15		5.4		12:15		55.0	0.1		
Volume	10	7	49	66	15	0	16	31	73	9	4	86	30	13	38	4	85	
Peak Factor				0.792				0.782				0.863					0.726	
Peak Hour Fro	m 02:00) PM to	05:45	PM - Peal	k 1 of 1													
Intersection			••••										1				- 1	
Volume	2	41	210	253	78	0	64	142	696	59	10	765	95	83	114	24	316	1476
Percent	0.8	16.2	83.0		54.9	0.0	45.1		91.0	7.7	1.3		30.1	26.3	36.1	7.6		
04:30	0	9	84	93	21	0	7	28	209	14	0	223	24	18	39	12	93	437
Volume Peak Factor																		0.844
High Int.	04:30	РМ			04:45	РМ			04:30	РМ			04:00	PM				4.4
Volume	0	9	84	93	34	0	33	67	209	14	0	223	34	23	30	7	94	
Peak Factor				0.680				0.530				0.858					0.840	
Peak Hour Fro	m 02:00	PM to	05:45	PM - Peal	k 1 of 1													
Ву	05:00				04:30	DМ			04:00	РМ			05:00	РМ				
Approach								.=-				-05			4	^^	400	
Volume	8	50	210	268	102	0	76	178	696	59	10	765	148	100 23.6	147 34.8	28 6.6	423	
Percent High Int.	3.0 05:45	18.7 PM	78.4		57.3 04:45	0.0 PM	42.7		91.0 04:30	7.7 PM	1.3		35.0 05:00		34.0	Ų.Ų		
Volume	00:40	28	82	116	34	C 141	33	67	209	14	0	223	45	32	37	8	122	
Peak Factor				0.578		Ĭ		0.664	,			0.858					0.867	
					•				•				•					

City of Rocky Mount
Engineering Dept.
One Government Restame: Arlington St.-Atlantic Ave. & Tarboro St

Rocky Mount, NCS26804de : 01421137 Start Date : 7/10/2003

								Date . 7),)			
							Page						
					Groups F	Printed- 1	- Unshifte	d					
		Atlanti	c Ave.			Arlingt				Tarbo		1	
			bound			Northt				Eastb	ouna Diabel	App. Total	Int. Total
 Start Time	Left	Thru	Right A	pp. Total	Left	Thru		App. Total	Left	Thru 1.0	1.0	App. Total	III. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0	40	1.0	1.0	- 1.0	20	74
07:00 AM	10	25	0	35	0	15	4	19	2	12 25	2	31	125
07:15 AM	10	32	0	42	0	46	6	52 58	4 2	25 17	5	24	119
07:30 AM	6	31	0	37	0	48 70	10 6	76	3	29	7	39	183
 07:45 AM	17_	51	0	68	0	179	26	205	11	83	20	114	501
Total	43	139	0	182	U	1/9	20	205	• • • • • • • • • • • • • • • • • • • •	00	20	,,,,	001
08:00 AM	17	40	0	57	0	48	9	57	1	21	3	25	139
08:15 AM	8	47	Ö	55	0	37	9	46	1	18	4	23	124
08:30 AM	7	25	0	32	0	26	3	29	1	26	1	28	89
08:45 AM	5	20	0	25	0	49	12	61	0	26 _	0	26	112
 Total	37	132	0	169	0	160	33	193	3	91	8	102	464
11:00 AM	16	31	0	47	0	58	14	72	7	58	10	75	194
11:15 AM	12	31	0	43	0	29	3	32	8	42	1	51	126
11:30 AM	11	43	Ō	54	0	50	14	64	8	56	2	66	184
11:45 AM	12	29	0	41	0	32	19	51	10	45	5	_60	152
 Total	51	134	0	185	0	169	50	219	33	201	18	252	656
40.00 PM	11	29	0	40	0	38	11	49	2	51	4	57	146
12:00 PM	9	29 38	0	47	0	50	4	54	1	35	i	37	138
12:15 PM	10	36 31	0	41	0	44	4	48	5	38	i	44	133
12:30 PM 12:45 PM	12	33	0	45	Õ	41	7	48	4	42	3	49	142
 Total	42	131	0	173	<u>_</u>	173	26	199	12	166	9	187	559
Total			-	•	·			·					400
04:00 PM	19	42	0	61	0	43	11	54	8	43	2	53	168
04:15 PM	15	36	0	51	0	24	32	56	6	78	1	85	192
04:30 PM	21	39	0	60	0	63	17	80	6	67	5	78	218 222
04:45 PM	28	37	0	65	0	57	16	73	12	65	7 15	84 300	800
Total	83	154	0	237	0	187	76	263	32	253	15	·	
05:00 PM	29	55	0	84	0	81	16	97	2	80	10	92	273
05:15 PM	21	38	0	59	0	56	19	75	4	83	10	97	231
05:30 PM	22	38	0	60	0	72	29	101	6	73	8	87	248
05:45 PM	22	36	0	58	0	57_	25	82	4	68	11	83	223 975
 Total	94	167	0	261	0	266	89	355	16	304	39	359	
Grand Total	350	857	0	1207	0	1134	300	1434	107	1098	109	1314	3955
Approh %	29.0	71.0	0.0		0.0	79.1	20.9		8.1	83.6	8.3		
Total %	8.8	21.7	0.0	30.5	0.0	28.7	7.6	36.3	2.7	27.8	2.8	33.2	
				ű.				·					

City of Rocky Mount
Engineering Dept.
One Government Readame: Arlington St.-Atlantic Ave. & Tarboro St
Rocky Mount, NCS27804de: 01421137
Start Date: 7/10/2003

	<u> </u>	Atlant	ic Ave. bound				ton St. bound			Tarbo Eastb			
Start Time	Left	Thru	Right Ap	p. Total	Left	Thru		App. Total	Left	Thru		App. Total	Int. Total
Peak Hour From 07	00 AM to 0							<u></u>	·				
	07:15 AM]				
Volume	50	154	0	204	0	212	31	243	10	92	17	119	566
Percent	24.5	75.5	0.0		0.0	87.2	12.8		8.4	77.3	14.3	20	400
07:45 Volume	17	51	0	68	0	70	6	76	3	29	7	39	183 0.773
Peak Factor	07.45.414				07:45 084				07:45 AM				0.773
High Int.	07:45 AM 17	51	0	68	07:45 AM 0	70	6	76	3	29	7	39	
Volume Peak Factor	17	JI	U	0.750		70	Ū	0.799			·	0.763	
Peak Hour From 07	':00 AM to 0	8:45 AM	- Peak 1 of 1	ŧ									
By Approach					07:15 AM				07:15 AM				
Volume	48	169	0	217	0	212	31	243	10	92	17	119	
Percent	22.1	77.9	0.0		0.0	87.2	12.8		8.4	77.3	14.3		
•	07:45 AM		_		07:45 AM	~~		70	07:45 AM	20	7	39	
Volume	17	51	0	68	0	70	6	76 0.799	3	29	,	0.763	
Peak Factor				0.798	l			0.199	1			0.700	
Peak Hour From 11		2:45 PM	- Peak 1 of 1	1	1				I			}	
Intersection Volume	51	134	0	185	0	169	50	219	33	201	18	252	656
Percent	27.6	72.4	0.0	100	0.0	77.2	22.8	2.0	13.1	79.8	7.1		
11:00 Volume	16	31	0	47	0	58	14	72	7	58	10	75	194
Peak Factor	,,												0.845
	11:30 AM				11:00 AM				11:00 AM				
Volume	11	43	0	54	0	58	14	72	7	58	10	75	
Peak Factor				0.856	İ			0.760				0.840	
Peak Hour From 11		2:45 PM	- Peak 1 of	1								1	
By Approach				405	11:00 AM	400	50	040	11:00 AM	204	40	252	
Volume	51	134	0	185	0	169 77.2	50 22.8	219	33 13.1	201 79.8	18 7.1	252	
Percent High Int.	27.6 11:30 AM	72.4	0.0		0.0 11:00 AM	11.2	22.0		11:00 AM	75.0	7.1		
Volume	11.30 AM	43	0	54	0	58	14	72	7	58	10	75	
Peak Factor	• • • • • • • • • • • • • • • • • • • •	40	Ü	0.856		-	• • •	0.760				0.840	
Peak Hour From 04	1:00 PM to 0	5:45 PM	- Peak 1 of	1									
Intersection					1								
Volume	94	167	0	261	0	266	89	355	16	304	39	359	975
Percent	36.0	64.0	0.0		0.0	74.9	25.1		4.5	84.7	10.9		070
05:00 Volume	29	55	0	84	0	81	16	97	2	80	10	92	273 0.893
Peak Factor	05-00 PM				05:30 PM				05:15 PM				0.093
	05:00 PM 29	55	0	84	05.30 FW	72	29	101		83	10	97	
Volume Peak Factor	29	33	Ū	0.777		12	20	0.879		30		0.925	
Peak Hour From 04	1:00 PM to 0	5:45 PM	- Peak 1 of 1	1									
By Approach					05:00 PM				04:45 PM				
Volume	99	169	0	268	0	266	89	355	24	301	35	360	
Percent	36.9	63.1	0.0		0.0	74.9	25.1		6.7	83.6	9.7		
	05:00 PM		_	<u>.</u> .	05:30 PM		•	4.5.	05:15 PM	-00	40	0.7	
Volume	29	55	0	84	0	72	29	101	4	83	10	97 0.928	
Peak Factor				0.798	I			0.879	1			0.820	

File Name: hill st.-western ave. & main s

Site Code : 01421107 Start Date : 7/22/2003

										Groups	Printe												
			V Main				147	Hill St					: Main irthbou				F	astbou	ind				
			uthbou					estbou				T		1						Ann	Excl	Inclu.	Int.
Start Time	Left	Thr	Rig ht	Pe ds	App. Total	Left	Thr	Rig ht	Pe ds	App. Total	Left	Thr	Rig ht	Pe ds	App. Total	Left	Thr u	Rig ht	Pe ds	App. Total	_ u.	Tota	Total
					- 10101	4.5				· Ottal	4.0		Į.			4.0		1.0	1.0		Total		
Factor 07:00 AM	1.0	1.0	1.0	1.0	2	1.0	1.0 18	1.0	1.0	24	1.0	1.0	1.0 0	1.0	1	1.0	1.0	0	3	٥	3	27	30
07:00 AM	0	4	ő	Ö	4	Ö	21	ő	Ö	21	ŏ	2	ŏ	ŏ	2	ő	ő	ŏ	ŏ	o l	ō	27	27
07:30 AM	ō	5	ō	Ŏ	5	2	30	4	0	36	0	2	0	0	2	0	0	0	2	0	2	43	45
07:45 AM	0	3	0	0	3	7	44	1_	2	52	0	4	0	0	4	0	0	0	1	0	3	59	62
Total	0	14	0	0	14	10	113	10	2	133	0	9	0	0	9	0	0	0	6	0	8	156	164
08:00 AM	0	7	1	0	8	2	29	4	3	35	2	3	0	1	5	0	0	0	2	0	6	48	54
08:15 AM	Ō	7	5	0	12	10	37	1	0	48	0	2	0	0	2	0	0	0	1	0	1	62	63
08:30 AM	0	10	2	0	12	5	44	4	0	53	0	7	0	1	7	0	0	0	1	0	2	72	74
08:45 AM	0	5	0_	3	5	8	44	2	<u>0</u> 3	54_	<u>1</u>	<u>5</u> 17	0	2	6 20	0	0	0	<u>1</u> 5	0	13	65 247	<u>69</u> 260
Total	0	29	8	3	37	25	154	11	3	190	3	17	v	2	20	U	U	U	5	U .	13	241	200
	•	-00	_		00		20		_	50	2					۱ ۵	0	0	_	0	8	86	94
11:00 AM 11:15 AM	0	22 12	6 6	2 1	28 18	7 9	39 41	4 1	0 3	50 51	2 4	6 6	0	1 0	8 10	0	0	0	5 0	0	4	79	83
11:30 AM	0	31	2	Ó	33	10	36	Ö	0	46	1	6	Ö	ő	7	ő	ő	Ö	ŏ	ŏ	ō	86	86
11:45 AM	ŏ	15	3	1	18	8	33	3	ō	44	1	3	ō	Ō	4	ō	ō	Ō	2	0	3	66	69
Total	0	80	17	4	97	34	149	8	3	191	8	21	0	1	29	0	0	0	7	0	15	317	332
12:00 PM	0	24	7	0	31	3	54	2	0	59	1	3	0	0	4	l 0	0	0	0	0	0	94	94
12:00 PM	ő	22	7	3	29	4	28	3	ŏ	35	3	8	ŏ	2	11	ő	ŏ	ŏ	Ŏ	ŏ	5	75	80
12:30 PM	ō	29	9	ō	38	2	39	Ō	0	41	5	3	0	0	8	0	0	0	2	0	2	87	89
12:45 PM	0	23	9	5	32	6	35	0	3	41	2	8	0	0	10	0	0	0	7	0	15	83	98
Total	0	98	32	8	130	15	156	5	3	176	11	22	0	2	33	0	0	0	9	0	22	339	361
																			_	_			
04:00 PM	0	39	9	0	48	11	43	5	2	59	0	5	0	1	5	0	0	0	0	0	3	112	115
04:15 PM	0	29	9	1	38	7	26 30	2	0	30	1 2	9 5	0	0	10 7	0	0	0	10 7	0	11 9	78 75	89 84
04:30 PM 04:45 PM	0	23 20	6 6	1 0	29 26	7	30	2	0 1	39 41	2	3	0	Ó	5	0	0	0	2	0	3	73	75
Total	0	111	30	2	141	27	130	12	3	169	5	22	- ŏ	2	27	ŏ	ŏ	ō	19	ŏ	26	337	363
	_											_	_	_	_		_	_	_	_			
05:00 PM	0	14	5	0	19	1	26	9	0	36	1	6	0	0	7	0	0	0	0	0	0	62 40	62 40
05:15 PM	0	6	4	0	10	1	24 21	1 2	0	26 23	1 2	3 1	0	0	4	0	0	0	6	0	6	40 47	53
05:30 PM 05:45 PM	0	20 14	1 2	0	21 16	0	27	2	0	29	0	Ó	0	0	0	a	0	Ö	2	ŏ	2	45	47
Total	0	54	12	- 0	66	2	98	14	ŏ	114	4	10	- 6	0	14	ő	ő	0	8	ŏ	8	194	202
	•			•		. –		• •	-				-	-		· -	-				·		
Grand Total	0	386	99	17	485	113	800	60	14	973	31	101	0	7	132	0	0	0	54	0	92	1590	1682
-		79.	20.			11.	82.	6.2			23.	76.	0.0			0.0	0.0	0.0					
Apprch %	0.0	6	4			6	2	Ų.Z			5	5	0.0			0.0	0.0	0.0					
Total %	0.0	24. 3	6.2		30.5	7.1	50. 3	3.8		61.2	1.9	6.4	0.0		8.3	0.0	0.0	0.0		0.0	5.5	94.5	
		J				l	9									I					ı		

File Name: hill st.-western ave. & main s

Site Code : 01421107 Start Date : 7/22/2003

			fain St.				II St. tbound				Main St. hbound			East	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	int. Total
eak Hour Fron			8:45 AM	- Peak 1	of 1				1							i	
Intersection Volume	08:00 A 0	107 29	8	37	25	154	11	190	3	17	0	20	0	0	0	0	247
Percent	0.0	78.4	21.6	01	13.2	81.1	5.8	150	15.0	85.0	0.0		0.0	0.0	0.0	Ĭ	
08:30				40	5		4	53	0	7	0	7	0	0	0	0	72
Volume	0	10	2	12) 3	44	4	53	"	,	U	′	"	U	U	· ·	
Peak Factor																	0.858
High Int.	08:15 A		_		08:45		_		08:30		•	_	6:45:00) AM		ĺ	
Volume	0	7	5	12	8	44	2	54	0	7	0	7					
Peak Factor				0.771	ţ			0.880	l			0.714	I			I	
eak Hour Fror			8:45 AM	- Peak 1	of 1												
By Approach					08:00 A				08:00				07:00 A		_	_	
Volume	0	29	8	37	25	154	11	190		17	0	20	0	0	0	0	
Percent	0.0	78.4	21.6		13.2	81.1	5.8		15.0	85.0	0.0		-	-	-		
High Int.	08:15 A		-	40	08:45 A		2	E 4	08:30	4м 7	0	7	-			-	
Volume Peak Factor	0	7	5	12 0.771	•	44	2	54 0.880	0	,	U	7 0.714	-	-	-	-	
eak Hour Fron			2:45 PM	- Peak 1	of 1				ı				1			ŀ	
Intersection Volume	12:00 F 0	-WI 98	32	130	15	156	5	176	11	22	0	33	0	0	0	0	339
Percent	0.0	75.4	24.6	100	8.5	88.6	2.8	170	33.3	66.7	0.0	00	0.0	0.0	0.0		300
12:00				0.4	ŀ			50								_	0.4
Volume	0	24	7	31	3	54	2	59	1	3	0	4	0	0	0	0	94
Peak Factor																	0.902
High Int.	12:30 F		_		12:00 F		_		12:15		_					1	
Volume Peak Factor	0	29	9	38 0.855	3	54	2	59 0.746	3	8	0	11 0.750					
					·				'				•			,	
'eak Hour Fron			2:45 PM	- Реак 1					12:00 F	20.4			11:00 A	. K.J		1	
By Approach Volume	12:00 F	и 98	32	130	11:15 A	164	6	200	12.001	-м 22	0	33	0	4IVI 0	0	0	
Percent	0.0	75.4	24.6	130	15.0	82.0	3.0	200	33.3	66.7	0.0	33		-	-	١	
High Int.	12:30 F		27.0		12:00 F		0.0		12:15 F		0.0		_			1	
Volume	0	29	9	38	3	54	2	59	3	8	0	11	-	_	_	-	
Peak Factor				0.855				0.847				0.750				-	
eak Hour Fron			5:45 PM	- Peak 1	of 1				1				1			1	
Volume	0	 111	30	141	27	130	12	169	5	22	0	27	0	0	0	o	337
Percent	0.0	78.7	21.3		16.0	76.9	7.1		18.5	81.5	0.0		0.0	0.0	0.0	1	•••
04:00	0	20	^	40	44	42	_	50			n	_	0	0	0	ام	440
Volume	0	39	9	48	11	43	5	59	0	5	0	5	"	U	v	0	112
Peak Factor																	0.752
High Int.			_		04:00 F		_		04:15		_						
Volume	0	39	9	48	11	43	5	59		9	0	10					
Peak Factor				0.734				0.716	!			0.675	I			ı	
eak Hour Fron			5:45 PM	- Peak 1													
By Approach				444	04:00 F		4.5	400	04:15 F		_		04:00 F			ا	
Volume	0	111	30	141	27	130	12	169	6	23	0	29	0	0	0	0	
Percent High Int.	0.0	78.7	21.3		16.0 04:00 F	76.9	7.1		20.7 04:15 F	79.3 M.	0.0			-	-		
Volume	04.00 F	7WI 39	9	48	11	-w 43	5	59	1	- IVI - 9	0	10		_	-	_	
Peak Factor	J	JJ	3	0.734		75	•	0.716		3	v	0.725	-	_	_	-	
				J., O T	ı			J., J	ı			J.1.20	1			I	

 $File\ Name\ : Franklin\ St.\ \&\ Thomas\ S$

Site Code : 01320112 Start Date : 5/27/2003

Groups	Printed-	Unshifted

			klin St. hbound				nas St. tbound	Printed-			nbound			East	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	0	46	0	46	0	15	0	15	0	0	0	0	0	0	0	0	61
07:15 AM	0	54	2	56	0	17	0	17	0	0	0	0	0	0	0	0	73
07:30 AM	0	49	3	52	6	44	0	50	0	0	0	0	0	0	0	0	102
07:45 AM	0	114_	2	116	9	50	0	59	0	0	0	0	0	0	0	0	175
Total	0	263	7	270	15	126	0	141	0	0	0	0	0	0	0	0	411
08:00 AM	0	122	3	125	10	58	0	68	0	0	0	0	0	0	0	0	193
08:15 AM	0	103	1	104	14	62	0	76	0	0	0	0	0	0	0	0	180
08:30 AM	0	90	3	93	7	53	0	60	0	0	0	0	0	0	0	0	153
08:45 AM	0	71	5	76	22	72	0	94	0	0	0	0	0	0	0	0	170 696
Total	0	386	12	398	53	245	0	298	0	0	0	0	0	U	U	υį	990
11:00 AM	0	76	7	83	20	101	0	121	0	0	0	0	0	0	0	0	204
11:15 AM	ō	52	7	59	21	102	Ō	123	0	0	0	0	0	0	0	0	182
11:30 AM	ŏ	62	4	66	28	101	0	129	0	0	0	0	0	0	0	0	195
11:45 AM	Ō	73	16	89	26	123	0	149	0	0	0	0	0	0	0	0	238
Total	0	263	34	297	95	427	0	522	0	0	0	0	0	0	0	0	819
12:00 PM	0	82	15	97	28	141	0	169	0	0	0	0	0	0	0	0	266
12:15 PM	0	78	9	87	25	108	0	133	0	0	0	0	0	0	0	0	220
12:30 PM	0	69	7	76	30	128	0	158	0	0	0	0	0	0	0	0	234
12:45 PM	0	82	12	94	32	104	0	136	0	0	0	0	0	0	0	0	230
Total	0	311	43	354	115	481	0	596	0	0	0	0	0	0	0	0	950
04:00 PM	0	88	12	100	23	90	0	113	0	0	0	0	0	0	Ō	0	213
04:15 PM	0	80	4	84	26	98	0	124	0	0	0	0	0	0	0	0	208
04:30 PM	0	65	9	74	22	131	0	153	0	0	0	0	0	0	0	0	227
04:45 PM	0	60	6	66	12	85	0	97	0	0	0	0	0	0	0	0	163
Total	0	293	31	324	83	404	0	487	0	0	0	0	0	0	0	0	811
05:0 0 PM	0	74	5	79	18	94	0	112	0	0	0	0	0	0	0	0	191
05:15 PM	0	72	9	81	20	137	0	157	0	0	0	0	0	0	0	0	238
05:30 PM	0	59	3	62	10	93	0	103	0	0	0	0	0	0	0	0	165
05:45 PM	0	60	0	60	7	78	0	85	0	0	0	0	0	0	0	0	145
Total	0	265	17	282	55	402	0	457	0	0	0	0	0	0	0	0	739
Grand Total	0	1781	144	1925	416	2085	0	2501	0	0	0	0	0	0	0	0	4426
Apprch %	0.0	92.5	7.5	40.5	16.6	83.4	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
Total %	0.0	40.2	3.3	43.5	9.4	47.1	0.0	56.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	

File Name : Franklin St. & Thomas S

Site Code : 01320112 Start Date : 5/27/2003

	<u>-</u>		klin St. nbound				nas St.		_	North	nbound			East	tbound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
eak Hour Fron			8:45 AM	- Peak 1	of 1												
Intersection			_		4.0	222	_	000		•	^			0	0	اه	701
Volume	0	429	9	438	40 15.2	223 84.8	0.0	263	0.0	0.0	0.0	0	0.0	0.0	0.0	١	701
Percent	0.0	97.9	2.1		15.2	04.0	0.0									_	
08:00 Volume	0	122	3	125	10	58	0	68	0	0	0	0	0	0	0	0	193
Peak Factor																1	0.908
High Int.	08:00 A	М			08:15 A	AM.			6:45:00	AM			6:45:00	AM (
Volume	0	122	3	125	14	62	0	76									
Peak Factor				0.876				0.865									
eak Hour Fron			8:45 AM	- Peak 1												1	
By Approach					08:00 /		_		07:00 A		_	_	07:00 A				
Volume	0	429	9	438	53	245	0	298	0	0	0	0	0	0	0	0	
Percent	0.0	97.9	2.1		17.8	82.2	0.0		-	-	-		-	-	-		
High Int.			3	405	08:45 /	1M 72	0	94	-				-	_	_	_	
Volume Peak Factor	0	122	3	125 0.876	22	12	U	0.793	-	•	-	-	•	_	_	-	
eak Hour Fron			2:45 PM		of 1				, I				I			1	
Intersection	11:45 A		47	349	109	E00	0	609	٥ ا	0	0	0	0	0	0	0	958
Volume Percent	0 0.0	302 86.5	47 13.5	349	17.9	500 82.1	0.0	009	0.0	0.0	0.0	U	0.0	0.0	0.0	0	500
12:00												_					
Volume	0	82	15	97	28	141	0	16 9	0	0	0	0	0	0	0	0	266
Peak Factor													ĺ				0.900
High Int.	12:00 F	М			12:00 F	PM											
Volume Peak Factor	0	82	15	97 0.899	28	141	0	169 0.901									
					l			0.301	l				ı			ı	
'eak Hour Fron			2:45 PM	- Peak 1													
By Approach				0.54	11:45 /			200	11:00 A		•		11:00 A		^	0	
Volume	0	311 87.9	43	354	109 17.9	500 82.1	0.0	609	0	0	0	0	0	0	0	0	
Percent	0.0 12:00 F		12.1		17.9 12:00 F		0.0			-	-		_	-	-		
High Int. Volume	12.00 F	1VI 82	15	97	28	141	0	169		_	_	_	_	_	_	_	
Peak Factor	Ü	ŲŽ.	15	0.912		171	v	0.901				-				-	
¹eak Hour Fror	~ 04:00 I	OLA to O	E∙45 DM	Dook 1	of 1												
Intersection			3.43 FIVE	- reak i					1				l			1	
Volume	04.50 F	271	29	300	72	447	0	519	0	0	0	0	0	0	0	0	819
Percent	0.0	90.3	9.7	000	13.9	86.1	0.0	0.0	0.0	0.0	0.0	•	0.0	0.0	0.0		
05:15				04				457			^	•		0	0	0	220
Volume	0	72	9	81	20	137	0	157	0	0	0	0	0	U	U	U	238
Peak Factor									ļ								0.860
High Int.	05:15 F				05:15 F												
Volume	0	72	9	81	20	137	0	157									
Peak Factor				0.926	ļ			0.826								1	
eak Hour Fron	n 04:00 l	PM to 0	5:45 PM	- Peak 1	of 1												
By Approach	04:00 F	M			04:30 F				04:00 F			_	04:00 F		_		
Volume	0	293	31	324	72	447	0	519	0	0	0	0	0	0	0	0	
Percent	0.0	90.4	9.6		13.9	86.1	0.0		-	-	-		-	-	-		
High Int.			12	100	05:15 F 20	137 137	0	157	_	_	_		_	_	_	_	
Volume Peak Factor	0	88	12	0.810		137	U	0.826	-	-	-	_	_	-	-	-	
, can actu				0.010	I			0.020	ı			- 1	ı			1	

 $File\ Name\ : Church\ St.\ \&\ Thomas\ S$

Site Code : 01421100 Start Date : 5/21/2003

Page No : 1

																1	age	110					
										Groups	Printe												
								omas					nurch :		:								
,		So	uthbol	und			W	estbo	und			No	rthbou	und			E	astbou	ind				
Start		Thr	Rig	Pe	App.		Thr	Rig	Pe	App.		Thr	Rig	Pe	App.		Thr	Rig	Pe	App.	Excl	inclu.	Int.
Time	Left	u	ht	ds	Total	Left	ü	ht	ds	Total	Left	u	ht	ds	Total	Left	u	ht	ds	Total	u.	Tota	Total
11116		i			TOTAL					10.0.		_				4.0					Total		
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		- 0	04	91
07:00 AM	0	0	0	0	0	0	22	6	0	28	16	47	0	0	63	0	0	0	0	0	0	91	96
07:15 AM	0	0	0	0	0	0	23	1	0	24	16	56	0	0	72	0	0	0	0	0	0	96 121	121
07:30 AM	0	0	0	0	0	0	36	4	0	40	17	64	0	0	81	0	0	0	0	0	0	156	156
07:45 AM	0	0	0	0	0	0	57	5	0	62	21	73	0	0	9 <u>4</u> 310	0	- 0	0_	-0	0	- 6	464	464
Total	0	0	0	0	0	0	138	16	0	154	70	240	U	U	310	U	U	U	U	U	U	404	404
20.00.411	_	_	•	^	0	۱ ۵	E 0	40	0	68	42	88	0	1	130	0	0	0	0	0	1	198	199
08:00 AM	0	0	0	0	0	0	58 56	10 2	0	58	32	75	ő	Ó	107	0	Ö	ŏ	ő	õ	o	165	165
08:15 AM	0	0	0	0	0	0	34	6	0	40	44	77	Ö	ŏ	121	ő	ŏ	ŏ	Õ	Ö	ő	161	161
08:30 AM	0	0	0	0	0	o	63	6	Ö	69	26	61	ŏ	ŏ	87	ŏ	ő	ŏ	Õ	ŏ	Ö	156	156
08:45 AM Total	- 0	- 0	- 6	- ö	0		211	24	0	235	144	301	0	1	445	ŏ	ō	ō	0	0	1	680	681
TOtal	U	U	·	U	·	, 0	411	24	Ü	200	,	00,	Ŭ	•	, , ,	, ,	•	•	•	- 1		• • •	
11:00 AM	0	0	0	0	0	0	58	8	0	66	35	76	0	0	111	0	0	0	0	0	0	177	177
11:15 AM	ŏ	ŏ	Ö	Õ	ō	0	66	11	Ō	77	38	70	0	0	108	0	0	0	0	0	0	185	185
11:30 AM	Ö	ō	Ŏ	ō	Õ	0	78	12	2	90	44	94	0	1	138	0	0	0	0	0	3	228	231
11:45 AM	ŏ	Ō	ō	1	Ō	0	77	9	1	86	57	76	0	0	133	0	0	0	0	0_	2	219	221
Total	0	0	Ō	1	0	0	279	40	3	319	174	316	0	1	490	0	0	0	0	0	5	809	814
12:00 PM	0	0	0	0	0	0	62	15	0	77	63	105	0	0	168	0	0	0	0	0	0	245	245
12:15 PM	0	0	0	0	0	0	67	9	0	76	61	95	0	0	156	0	0	0	0	0	0	232	232
12:30 PM	0	0	0	0	0	0	51	8	0	59	48	84	0	0	132	0	0	0	0	0	0	191	191
12:45 PM	0	0	0	0	0	0	59	12	0	71	61	95	0	0	156	0	0	0	0	0	0	227	227
Total	0	0	0	0	0	0	239	44	0	283	233	379	0	0	612	0	0	0	0	0	0	895	895
	_	_	_	_	_		-00		_	00	1 40	00		^	100		Λ	٥	4	0	l 1	188	189
04:00 PM	0	0	0	0	0	0	66	14	0	80	40	68	0	0	108	0	0	0	1 0	0	6	235	235
04:15 PM	0	0	0	0	0	0	73	6	0	79	56	100	0	0	156	0	0	0	0	0	0	210	210
04:30 PM	0	0	0	0	0	0	74	13	0	87	41	82	0	0	123	_	-	0	0	0	2	235	237
04:45 PM	0	0	0	1	0	0	73	9	1_	82	38_	115 365	0	<u>0</u> 0	153 540	0	0	0	1	0	3	868	871
Total	0	0	0	1	0	0	286	42	1	328	175	365	U	U	540	U	v	U	1	U	1 3	000	071
05.00 014	_	_			^	1 0	97	16	1	113	65	179	0	1	244	0	0	0	0	0	2	357	359
05:00 PM	0	0	0	0	0	0	71	9	0	80	51	104	Ö	ó	155	ő	Ö	Ö	ő	ŏ	ō	235	235
05:15 PM	0	0	0	0	0	0	58	22	0	80	40	93	Õ	0	133	ő	Ö	Õ	Ö	ő	ő	213	213
05:30 PM	0	0	0	0	0	0	39	10	0	49	27	72	0	0	99	Ö	Ö	Ö	ő	ŏ	l ŏ	148	148
05:45 PM	0	0	0	0	- 0		265	57	1	322	183	448	- <u>0</u>	1	631	0	0	0	0	ő	2	953	955
Total	0	0	0	U	U	, 0	200	21	1	ŞZZ	1 100	440	U	'	031	1 0	U	U	U	U		500	000
Ceand						1	1.11				I	204				I					1		
Grand Total	0	0	0	2	0	0	141 8	223	5	1641	979	9	0	3	3028	0	0	0	1	0	11	4669	4680
TOTAL							86.	13.			32.	67.									-		
Apprch %	0.0	0.0	0.0			0.0	4	6			3	7	0.0			0.0	0.0	0.0			1	<u> </u>	
							30.				21.	43.										00.0	Ī
Total %	0.0	0.0	0.0		0.0	0.0	4	4.8		35.1	0	9	0.0		64.9	0.0	0.0	0.0		0.0	0.2	99.8	1
						1					, ,	•				,					٠.		_ \

Total without Peds -

File Name: Church St. & Thomas S Site Code: 01421100

Start Date : 5/21/2003

	_	South	bound	· · ·		-	nas St. tbound				rch St. hbound	_		East	tbound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
eak Hour Fron			8:45 AM	- Peak 1	of 1								 I			1	
Intersection			_	_		005		000	420	242	0	452	0	0	0	0	680
Volume	0	0	0	0	0	205	23	228	139 30.8	313 69.2	0 0.0	452	0.0	0.0	0.0	•	000
Percent	0.0	0.0	0.0		0.0	89.9	10.1		30.6	09.2	0.0		-				
08:00 Volume	0	0	0	0	0	58	10	68	42	88	0	130	0	0	0	0	198 0.859
Peak Factor					00.00				08:00	A B.4			6:45:00	LAA.			0.009
High Int.			^	0	08:00 A	ъм 58	10	68	42	-1VI 88	0	130	0.43.00	7.7.141			
Volume Peak Factor	0	0	0	U		50	10	0.838		00	J	0.869				Į	
eak Hour Fron			8:45 AM	- Peak 1												I	
By Approach	07:00 A	λM			08:00 A				07:45		_	450	07:00				
Volume	0	0	0	0		211	24	235	139	313	0	452	0	0	0	0	
Percent	-	-	-		0.0	89.8	10.2		30.8	69.2	0.0		-	-	-	ŀ	
High Int.	-				08:45 /		_		08:00			400	-				
Volume Peak Factor	-	-	•	-	0	63	6	69 0.851	42	88	0	130 0.869	-	-	-	-	
eak Hour Fron			2:45 PM	- Peak 1	of 1				ı				ı			i	
Intersection			^	0	0	284	45	329	225	370	0	595	0	0	0	0	924
Volume	0	0.0	0 0.0	U	0.0	86.3	13.7	329	37.8	62.2	0.0	333	0.0	0.0	0.0	ŭ	V - .
Percent	0.0	0.0	0.0		0.0		13.7										215
12:00 Volume	0	0	0	0	0	62	15	77	63	105	0	168	0	0	0	0	245 0.943
Peak Factor					11120				12:00	οм							0.040
High Int.	^	•	0	0	11:30 /	-\м 78	12	90		105	0	168				İ	
Volume Peak Factor	0	0	U	U	"	10	14	0.914		100	Ū	0.885					
eak Hour Fron	n 11:00	AM to 1	2:45 PM	- Peak 1	of 1												
By Approach			_,		11:15	AΜ			12:00	PM			11:00 /	AΜ			
Volume	0	0	0	0	0	283	47	330	233	379	0	612	0	0	0	0	
Percent	-	-	-		0.0	85.8	14.2		38.1	61.9	0.0		-	-	-		
High Int.	-				11:30 /	ΑМ			12:00	PM [*]			-				
Volume	-	-	-	-	0	78	12	90	63	105	0	168	-	-	-	-	
Peak Factor				-				0.917				0.911				-	
eak Hour Fron			5:45 PM	- Peak 1	of 1											I	
Volume	0	0	0	0	0	299	56	355	194	491	0	685	0	0	0	0	1040
Percent 05:00	0.0	0.0	0.0	^	0.0	84.2	15.8 16	113	28.3 65	71.7 179	0.0	244	0.0	0.0		0	357
Volume Peak Factor	0	0	0	0	0	97	10	113	0.5	119	v	277			J		0.728
High Int.					05:00 8	PM			05:00	PM							
Volume	0	0	0	0	0	97	16	113	65	179	0	244					
Peak Factor								0.785				0.702				1	
eak Hour Fron			5:45 PM	- Peak 1	of 1	~**			104.45				Loaion	214		1	
By Approach			_	_	04:30		47	200	04:45		^	685	04:00 1	-м О	0	0	
Volume	0	0	0	0	1	315		362	194 28.3	491 71.7		COQ	1	-	-	J	
Percent		-	-		0.0	87.0	13.0		05:00		0.0		1	-	-		
High Int.	-				05:00 1	PM 97	16	113		- 179	0	244	_	-	_	-	
Volume Peak Factor	-	-	-	-		91	10	0.801		113	J	0.702				-	

File Name : Franklin St. & Sunset Ave

Site Code : 01321108 Start Date : 5/27/2003

Groups	Printed-	Unshifted

_							Groups	Printed-	Unshift	ed							
			klin St.												et Ave.		
		Sout	nboun <u>d</u>			Wes	tbound			North	bound			Easi	bound	- Ama	laŭ.
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Factor	1.0	1.0	1.0	10.0.	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	6	35	0	41	0	0	0	0	0	0	0	0	0	24	4	28	69
07:15 AM	14	52	0	66	0	0	0	0	0	0	0	0	0	32	13	45	111
07:30 AM	6	50	0	56	0	0	0	0	0	0	0	0	0	43	16	59	115
07:45 AM	15	95	0	110	0	0	0	0	0	0	0	0	0	59	21	80	190
Total	41	232	0	273	0	0	0	0	0	0	0	0	0	158	54	212	485
08:00 AM	12	74	0	86	0	0	0	0	0	0	0	0	0	82	18	100	186
08:15 AM	17	80	0	97	0	0	0	0	0	0	0	0	0	75	20	95	192
08:30 AM	16	65	0	81	0	0	0	0	0	0	0	0	0	72	20	92	173
08:45 AM	18_	50	0	68	_ 0	0	0	0	0_	0	0	0	0	65	27	92	160
Total	63	269	0	332	0	0	0	0	0	0	0	0	0	294	85	379	711
11:00 AM	34	56	0	90	0	0	0	0	0	0	0	0	0	101	31	132	222
11:15 AM	26	65	ŏ	91	ō	ŏ	ō	o l	0	0	0	0	0	87	22	109	200
11:30 AM	21	62	ō	83	Ō	Ō	Ö	0	0	0	0	0	0	94	18	112	195
11:45 AM	29	110	0	139	0	0	0	0	0	0	0	0	0	112	24	136	275
Total	110	293	0	403	0	0	0	0	0	0	0	0	0	394	95	489	892
12:00 PM	35	80	0	115	0	0	0	0	0	0	0	0	0	96	39	135	250
12:15 PM	35	84	0	119	0	0	0	0	0	0	0	0	0	103	31	134	253
12:30 PM	30	89	0	119	0	0	0	0	0	0	0	0	0	119	31	150	269
12:45 PM	34	99	0	133	0.	0	0	0	0	0	0	0	0	102 420	22 123	124	257 1029
Total	134	352	0	486	0	0	0	0	0	0	0	0	0	420	123	543	1029
04:00 PM	13	40	0	53	0	0	0	o	0	0	0	0	0	74	16	90	143
04:15 PM	27	66	ō	93	Ō	0	Ō	0	0	0	0	0	0	69	19	88	181
04:30 PM	24	60	0	84	0	0	0	0	0	0	0	0	0	97	20	117	201
04:45 PM	25	69	0	94	0	0	0	0	0	0	0	0	0	97	23	120	214
Total	89	235	0	324	0	0	0	0	0	0	0	0	0	337	78	415	739
05:00 PM	24	118	0	142	0	0	0	0	0	0	0	0	0	54	25	79	221
05:15 PM	21	68	0	89	0	0	0	0	0	0	0	0	0	93	14	107	196
05:30 PM	17	71	0	88	0	0	0	0	0	0	0	0	0	82	17	99	187
05:45 PM	41	122	0	163	0	<u>0</u>	0	0	0	0	0	0	0	109 338	24 80	133	296
Total	103	379	0	482	0	0	0	0	0	0	0	0	0	338	80	418	900
Grand Total	540	1760	0	2300	0	0	0	0	0	0	0	0	0	1941	515	2456	4756
Apprch %	23.5	76.5	0.0		0.0	0.0	0.0	2.5	0.0	0.0	0.0		0.0	79.0	21.0	-4.6	
Total %	11.4	37.0	0.0	48.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	40.8	10.8	51.6	

File Name: Franklin St. & Sunset Ave

Site Code : 01321108 Start Date : 5/27/2003

			klin St.	_		Wes	tbound			North	nbound				et Ave. tbound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
eak Hour Fron			8:45 AM	- Peak 1	of 1				ı				1			1	
Intersection			•	274			^	^	0	0	0	0	0	288	79	367	741
Volume	60 16.0	314 84.0	0.0	374	0.0	0.0	0.0	0	0.0	0.0	0.0	U	0.0	78.5	21.5	307	741
Percent 08:15		04.0	0.0		1				1							-	
Volume	17	80	0	97	0	0	0	0	0	0	0	0	0	75	20	95	192
Peak Factor																	0.965
High Int.	07:45 A	М			6:45:00	AM (6:45:00	AM			08:00 A	M			
Volume	15	95	0	110	0	0	0	0	0	0	0	0	0	82	18	100	
Peak Factor				0.850												0.918	
'eak Hour Fron			8:45 AM	- Peak 1												ı	
By Approach			_		07:00 A			_	07:00 A		•	•	08:00 A		05	270	
Volume	60	314	0	374	0	0	0	0	0	0	0	0	0	294	85	379	
Percent	16.0	84.0	0.0		-	-	-		-	-	-		0.0 08:00 A	77.6	22.4		
High Int. Volume	07:45 A 15	.м 95	0	110	-				-			_	08.00 2	82	18	100	
Peak Factor	15	90	U	0.850	-	-	-	-		·	•	-	"	02.	10	0.948	
eak Hour Fron			2:45 PM	- Peak 1	of 1				I				I			ŀ	
Volume	129	363	0	492	0	0	0	0	0	0	0	0	0	430	125	555	1047
Percent	26.2	73.8	0.0	402	0.0	0.0	0.0	•	0.0	0.0	0.0	·	0.0	77.5	22.5		
11:45				400				_				•				400	075
Volume	29	110	0	139	0	0	0	0	0	0	0	0	0	112	24	136	275
Peak Factor																	0.952
High Int.	11:45 A												12:30 F				
Volume Peak Factor	29	110	0	139 0.885	0	0	0	0	0	0	0	0	0	119	31	150 0.925	
eak Hour Fror	n 11:00 A	AM to 1	2·45 PM	- Peak 1	of 1												
By Approach					11:00 /	λM			11:00 A	M			11:45 A	ιM		1	
Volume	129	363	0	492	0	0	0	0	0	0	0	0	0	430	125	555	
Percent	26.2	73.8	0.0		-	_	_		-	-	-		0.0	77.5	22.5		
High Int.	11:45 A	M			-				-				12:30 F	M			
Volume	29	110	0	139	-	-	-	-	-	-	-	-	0	119	31	150	
Peak Factor				0.885	İ			-				-	ŀ			0.925	
eak Hour Fron			5:45 PM	- Peak 1	of 1				I				I			1	
Volume	103	379	0	482	0	0	0	0	0	0	0	0	0	338	80	418	900
Percent	21.4	78.6	0.0	402	0.0	0.0	0.0	U	0.0	0.0	0.0	U	0.0	80.9	19.1	410	300
05:45								_				_					
Volume	41	122	0	163	0	0	0	0	0	0	0	0	0	109	24	133	296
Peak Factor																f	0.760
High Int.	05:45 P	M											05:45 F	M			
Volume	41	122	0	163	0	0	0	0	0	0	0	0	0	109	24	133	
Peak Factor				0.739												0.786	
eak Hour Fron	n 04:00 F	M to 0	5:45 PM	- Peak 1	of 1												
By Approach		М			04:00 F				04:00 F				04:30 P			1	
Volume	103	379	0	482	0	0	0	0	0	0	0	0	0	341	82	423	
Percent	21.4	78.6	0.0		-	-	-		-	-	-		0.0	80.6	19.4		
High Int.			•	400	-				-				04:45 P		00	400	
Volume	41	122	0	163	-	-	-	-	-	-	-	-	0	97	23	120 0.881	
Peak Factor				0.739	I			+	!			-				0.001	

File Name: Church St. & Sunset Av. Site Code: 01421101

Start Date : 5/21/2003

Int. Total 91 127 149 181 548 198 180 188 164
91 127 149 181 548 198 180 188
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127 149 181 548 198 180 188
127 149 181 548 198 180 188
149 181 548 198 180 188
181 548 198 180 188
548 198 180 188
180 188
180 188
188
164
730
202
181
210
211
804
251
254
230
216
951
234
248 219
240
941
•
281
238
193
143
855
4829

File Name: Church St. & Sunset Ave

Site Code : 01421101 Start Date : 5/21/2003

			rch St.				et Ave. tbound				rch St. hbound			-	et Ave.		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
eak Hour Fron	n 07:00	AM to 0	8:45 AM		of 1		·····				· · · · · · · · · · · · · · · · · · ·						
Intersection	07:45 /	MA													_		
Volume	0	0	0	0	0	0	0	0	0	400	30	430	115	200	0	315	745
Percent	0.0	0.0	0.0		0.0	0.0	0.0		0.0	93.0	7.0		36.5	63.5	0.0		
08:00	0	0	0	0	0	0	0	0	l o	105	11	116	35	46	0	81	197
Volume	U	U	U	U		·	U	•	"	100	• •	110	00	-10	•	٠.	
Peak Factor																	0.945
High Int.	6:45:00) AM			6:45:00) AM			08:00 A				07:45 A				
Volume	0	0	0	0	0	0	0	0	0	105	11	116	21	63	0	84	
Peak Factor]							0.927				0.938	
eak Hour Fron			8:45 AM	- Peak 1	of 1												
By Approach	07:00 /	AΜ			07:00 A	٨M			07:45 /	AM			07:45 A	M			
Volume	0	0	0	0	0	0	0	0	0	400	30	430	115	200	0	315	
Percent	-	-	-		-	-	-		0.0	93.0	7.0		36.5	63.5	0.0		
High Int.	-				-				08:00 A	MΑ			07:45 A	M			
Volume	-	-	-	-	-	-	-	-	0	105	11	116	21	63	0	84	
Peak Factor				-				-				0.927				0.938	
eak Hour Fron			2:45 PM	- Peak 1	of 1				1				t			1	
Intersection	12:00 F							_	_				l		_		
Volume	0	0	0	0	0	0	0	0	0	446	40	486	114	331	0	445	931
Percent	0.0	0.0	0.0		0.0	0.0	0.0		0.0	91.8	8.2		25.6	74.4	0.0	i	
12:15	0	0	0	0	0	0	0	0	o	130	9	139	27	82	0	109	248
Volume	Ū	U	v	J	"		v	·			•	100		-	-	100	
Peak Factor																	0.939
High Int.									12:15 F				12:30 F				
Volume	0	0	0	0	0	0	0	0	0	130	9	139	27	93	0	120	
Peak Factor					ļ							0.874				0.927	
'eak Hour Fron	n 11:00	AM to 1	2:45 PM	- Peak 1	of 1												
By Approach	11:00 /	٩M			11:00 A	λM			11:30 A	AΜ			11:45 A				
Volume	0	0	0	0	0	0	0	0	0	450	39	489	108	339	0	447	
Percent	-	-	-		-	-	-		0.0	92.0	8.0		24.2	75.8	0.0		
High Int.	-				-				12:15 F				12:30 F				
Volume	-	-	-	-	-	-	-	-	0	130	9	139	27	93	0	120	
Peak Factor				-	}			-				0.879				0.931	
eak Hour Fron			5:45 PM	- Peak 1	of 1												
Intersection	04:15 l	PM														1	
Volume	0	0	0	0	0	0	0	0	0	509	39	548	97	332	0	429	977
Percent	0.0	0.0	0.0		0.0	0.0	0.0		0.0	92.9	7.1		22.6	77.4	0.0		
05:00	0	0	0	0	٥	0	0	0	0	163	15	178	21	81	0	102	280
Volume	U	U	v	U	"	U	v	J		100	10	173	^'	0.	•	.02	
Peak Factor																	0.872
High Int.									05:00 F				04:45 F				
Volume	0	0	0	0	0	0	0	0	0	163	15	178	24	89	0	113	
Peak Factor												0.770				0.949	
eak Hour Fron	n 04:00	PM to 0	5:45 PM	- Peak 1	of 1												
By Approach					04:00 F	PM			04:15 F	PM			04:00 F			i	
Volume	0	0	0	0	0	0	0	0	0	509	39	548	100	330	0	430	
Percent	-	-	-		-	-	_		0.0	92.9	7.1		23.3	76.7	0.0	l	
High Int.	-				-				05:00 F	PM			04:45 F	M			
Volume	-	-	-	-	-	-	-	-	0	163	15	178	24	89	0	113	
Peak Factor				-	1			-	1			0.770	I			0.951	

City of Rocky Mount Engineering Dept. One Government Plaza

One Government Plaza Rocky Mount, NC 27804

File Name: Franklin St. & Western Ave

Site Code : 01321107 Start Date : 5/27/2003

Page No : 1

Groups Printed- Unshifted

								s Printed-	Unshift	ed							
			klin St.				ern Ave					1					
		Sout	hbound			Wes	tbound			Norti	hbound			Eas	tbound	A	Ind
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	0	37	3	40	3	9	0	12	0	0	0	0	0	0	0	0	52
07:15 AM	0	50	0	50	9	21	0	30	0	0	0	0	0	0	0	0	80
07:30 AM	0	61	2	63	4	49	0	53	0	0	0	0	0	0	0	0	1 16
07:45 AM	0	98	2	100	2	22	0	24	0	. 0	0	0	0	0	0	0	124
Total	0	246	7	253	18	101	0	119	0	0	0	0	0	0	Ö	0	372
08:00 AM	0	80	3	83	7	28	0	35	0	0	0	0	0	0	0	0	118
08:15 AM	0	90	1	91	15	20	0	35	0	0	0	0	0	0	0	0	126
08:30 AM	0	77	1	78	7	16	0	23	0	0	0	0	0	0	0	0	101
08:45 AM	0	80	1	81	11	23	0	34	0	0	0	0	0	0	0	0	115
Total	0	327	6	333	40	87	0	127	0	0	0	0	0	0	0	0	460
11:00 AM	0	63	1	64	25	27	0	52	0	0	0	0	0	0	0	0	116
11:15 AM	ő	81	5	86	19	27	ŏ	46	ō	Ŏ	ō	ō	Ö	0	0	Ō	132
11:30 AM	ő	87	2	89	14	26	ŏ	40	Ö	ō	ŏ	ŏ	ō	Ō	Ō	0	129
11:45 AM	Ö	89	8	97	15	37	ō	52	ō	Ō	ō	ō	0	0	0	Ó	149
Total	Ö	320	16	336	73	117	0	190	0	0	0	0	0	0	0	0	526
12:00 PM	0	93	9	102	16	32	0	48	0	0	0	0	0	0	0	0	150
12:15 PM	0	109	7	116	15	47	0	62	0	0	0	0	0	0	0	0	178
12:30 PM	0	101	8	109	13	44	0	57	0	0	0	0	0	0	0	0	166
12:45 PM	0	95	8	103	17	55	0	72	0	0	0	0	. 0	0	0	0	175
Total	0	398	32	430	61	178	0	239	0	0	0	0	0	0	0	0	669
04: 00 PM	0	90	1	91	14	29	0	43	0	0	0	0	0	0	0	0	134
04:15 PM	0	96	3	99	12	31	0	43	0	0	0	0	0	0	0	0	142
04:30 PM	0	77	2	79	10	34	0	44	0	0	0	0	0	0	0	0	123
04:45 PM	0	80	4	84	9	23	0	32	0	0	0	0	0	0	0	0	116
Total	0	343	10	353	45	117	0	162	0	0	0	0	0	0	0	0	515
05:00 PM	0	127	6	133	18	49	0	67	0	0	0	0	0	0	0	0	200
05:15 PM	0	79	2	81	10	44	0	54	0	0	0	0	0	0	0	0	135
05:30 PM	0	79	3	82	13	25	0	38	0	0	0	0	0	0	0	0	120
05:45 PM Total	0	<u>56</u> 341	4 15	60 356	3 44	15 133	0	18 177	0	0	0	0	0	0	0	0	78 533
Grand Total	0	1975	86	2061	281	733	0	1014	0	0	0	0	0	0	0	0	3075
Apprch %	0.0	95.8	4.2		27.7	72.3	0.0		0.0	0.0	0.0		0.0	0.0	0.0	- 1	
Total %	0.0	64.2	2.8	67.0	9.1	23.8	0.0	33.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
/0	0.0			Ţ .												1	

City of Rocky Mount Engineering Dept. One Government Plaza

One Government Plaza Rocky Mount, NC 27804 File Name: Franklin St. & Western Ave

Site Code : 01321107 Start Date : 5/27/2003

Start Time Left Timu Right App Total Tot				klin St. nbound				ern Ave	-		North	nbound			East	bound		
Intersection 07-30 AM Volume 0 329 8 337 28 119 0 147 0 0 0 0 0 0 0 0 0	Start Time	Left				Left		I "I		Left	Thru	Right		Left	Thru	Right		Int. Total
Volume	eak Hour Fron	n 07:00 /	AM to 0	8:45 AM		of 1								<u> </u>			1	
Percent 1:00 AM to 12:45 PM - Peak 1 of 1 1245 PM - Peak Factor From 11:00 AM to 12:45 PM - Peak 1 of 1 1245 PM - Peak Factor Highlint 12:00 PM - Peak Factor Highlint 10:00 PM to 12:45 PM - Peak 1 of 1 1246 PM - Peak Factor From 11:00 AM to 12:45 PM - Peak 1 of 1 1246 PM - Peak Factor From 11:00 AM to 12:45 PM - Peak 1 of 1 1246 PM - Peak Factor From 11:00 AM to 12:45 PM - Peak 1 of 1 1246 PM - Peak Factor From 11:00 AM to 12:45 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 12:15 PM - Peak 1 of 1 1246 PM - Peak Factor Highlint 10:00 PM - O 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				_				_	4.47	_	_	•		_	^	Ω	ام	484
126		-			337				147				U			_	٧I	404
Volume Peak Factor		0.0	97.6	2.4		19.0	81.0	0.0		0.0	0.0	0.0		0.0	0.0			
Peak Factor High Int. 12/15 PM Peak Tactor 0.0 96.2 3.8 3.0 8.9 18/16 1.0 0.0 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0	90	1	91	15	20	0	35	0	0	0	0	0	0	0	0	126
Peak Factor 100 OM to 12/45 PM - Peak of 1 12/45 PM Peak Factor 100 OM to 12/45 PM - Peak of 1 12/45 PM Peak Factor O 100 OM to 12/45 PM - Peak of 1 12/45 PM O 15/45 PM O 10/45 PM O 1																		0.960
Volume		07.45.6				07:20 4	R4			6:45:00	ΔRA			6:45:00) AM			0.000
Peak Factor 1.00 AM to 08:45 AM - Peak 1 of 1 1.00 AM - Peak 1 of 1 1.00 AM - Peak				2	100			n	53	0.40.00	CIVI			0.40.00	, ,		İ	
Value		U	90	2		*	70	•]	
By Approach 07-45 AM	Peak Facior				0.040	i			0.000	ı				!			'	
By Approach 07-45 AM	'eak Hour Fron	n 07:00 a	AM to 0	8:45 AM	- Peak 1	of 1												
Volume 0 345 7 352 28 119 0 147 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0							M			07:00 A	AM .			07:00				
Peak Factor 0 98 2 000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				7	352	28	119	0	147	0	0	0	0	0	0	0	0	
Volume	Percent	0.0	98.0	2.0				0.0		-	-	-		-	-	-	į	
Peak Factor	High Int.	07:45 A	M			07:30 A	M			-				-				
Peak Hour From 11:00 AM to 12:45 PM - Peak 1 of 1 Intersection 12:00 PM Volume 0 398 32 430 61 178 0 239 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 178 Percent 0.0 92.6 7.4 25.5 74.5 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Volume	0	98	2		-	49	0		-	-	-	-	-	-	-	-	
Intersection 12:00 PM	Peak Factor				0.880	j			0.693	İ			-	l			-	
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By Approach 12:00 PM	Peak Factor				0.927				0.830	ļ							I	
By Approach 12:00 PM		44.00		0.45 DM	Danis 4	-6.4												
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	Peak Factor				0.742				0.735	ļ			-	I			-	

City of Rocky Mount Engineering Dept. One Government Plaza

Rocky Mount, NC 27804

Groups Printed- Unshifted

File Name: Church St. & Western Avo

Site Code : 01321102 Start Date : 5/20/2003

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07:15 AM	0	0	0	0	0	0	23	4	1	27	2							0	0	ŏ	2	121	123
07:30 AM	0	0	0	2	0	0	36	9	0	45	7	69	0	0	76	0	0		_				
07:45 AM	0	0	0	0_	0	0	17	17	0	34	8	92	0	0	100	0	0	0	0	0	0	134	134
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08:00 AM	0	0	0	0	0	0	39	21	0	60	12	83	0	1	95	0	0	0	0	0	1	155	156
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Total	0	0	0	Ó	0	0	118	84	0	202	49	462	0	0	511	U	0	U	U	U	U	/13	113
05:00 PM	0	0	0	0	0	0	35	35	0	70	7	195	0	0	202	0	0	0	0	0	0	272	272
05:15 PM	0	0	0	0	0	0	36	10	0	46	4	171	0	0	175	0	0	0	0	0	0	221	221
05:30 PM	ō	ō	Ŏ	ō	ŏ	Ō	31	16	Ō	47	13	122	0	0	135	0	0	0	0	0	0	182	182
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Apprch %	0.0	0.0	0.0			0.0	0	0			9.7	3	0.0			0.0	0.0	0.0					
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Total %	0.0	0.0	0.0		0.0	0.0	3	7		31.1	6.7	3	0.0		68.9	0.0	0.0	0.0		0.0	0.3	99.7	

File Name: Church St. & Western Avo

Site Code : 01321102 Start Date : 5/20/2003

Start Time			South	abound				ern Ave.				rch St. hbound			East	bound		
Intersection 08:00 AM		1	Thru	Right	Total		Thru	Right		Left	Thru	Right		Left	Thru	Right		int. Total
Volume 0 0 0 0 0 0 0 104 97 201 36 328 0 364 0 0 0 0 0 5 56 Parcent 0 0 0 0 0 0 517 48.3 99 90.1 0 0 0 0 0 0 0 0 0 566 Molecular Minimals (145.00 AM Volume 0 0 0 0 0 0 0 0 39 21 60 12 83 0 95 0 0 0 0 0 0 0 155 99.8 Peak Factor High Int. 645.00 AM Volume 0 0 0 0 0 0 0 39 21 60 08.30 AM Volume 0 0 0 0 0 0 0 0 0 0 39 21 60 08.30 AM Volume 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				8:45 AM	- Peak 1	of 1								1			1	
Petcent 0,0 0,0 0,0 0,0 0,0 0,0 51,7 48,3 9,9 90,1 0,0 9,0 0,0 0,0 0,0 0,0 0,0 0,0 0,0 0,0				_	_	_						_	201	_	_	_		505
08:00 Volume		-	_		0	_			201				364	_	-		0	505
Volume		0.0	0.0	0.0		0.0	51.7	48.3		9.9	90.1	0.0		0.0	0.0	0.0		
Volume		0	0	0	0	0	39	21	60	12	83	0	95	0	0	0	0	155
New Form 11:00 AM to 12:45 PM - Peak 1 of 1		•	•	•	•	_						-						0.044
Volume														0.45.00				0.911
Peak Factor	~			_								_		6:45:00	AM			
**Pask Hour From 07:00 AM to 08:45 AM - Peak 1 of 1 By Approach 07:00 AM Yolume 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0	0	0	0	0	39	21		1	91	U		ł			1	
By Approach 07:00 AM	Peak Factor					1			0.838	1			0.929	Ī			1	
Volume Peak Factor	'eak Hour From	n 07:00 /	AM to 0	8:45 AM	- Peak 1	of 1												
Percent High Int. Volume 0.0 51.7 48.3 08.00 AM 07.45 AM 07.45 AM 07.45 AM 08.00 AM 0	By Approach	07:00 A	M			08:00 A	M											
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Volume 0 39 21 60 8 92 0 100	Percent	-	-	-				48.3				0.0		-	-	-		
Peak Hour From 11:00 AM to 12:45 PM - Peak 1 of 1 Intersection 11:15 AM Volume 0 0 0 0 0 0 0 162 104 266 61 464 0 525 0 0 0 0 0 0 791 Percent 0.0 0.0 0.0 0 0 0 60.9 39.1 16.6 88.4 0.0 525 0 0 0 0 0 0 791 2:00 0 0 0 0 0 0 36 18 54 32 136 0 168 0 0 0 0 0 222 Peak Factor High Int. Volume 0 0 0 0 0 0 11:30 AM Volume 0 0 0 0 0 0 162 104 266 8.87 32 136 0 168 0 0 0 0 0 0 891 Peak Hour From 11:00 AM to 12:45 PM - Peak 1 of 1 By Approach 11:00 AM Volume 0 0 0 0 0 162 104 266 75 471 0 546 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	High Int.	-				08:00 A	M			07:45				-				
Peak Hour From 11:00 AM to 12:45 PM - Peak 1 of 1 **Intersection 11:15 AM Volume 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Volume	-	-	-	-	0	39	21			92	0		-	-	-	-	
Intersection	Peak Factor				-				0.838	1			0.935				-	
Volume 0 0 0 0 0 0 162 104 266 61 464 0 525 0 0 0 0 0 791 Percent 0.0 0.0 0.0 0.0 0 0 0 0 60.9 39.1 11.6 88.4 0.0 525 0 0 0 0 0 0 0 222 Volume Peak Factor High Int. Volume 0 0 0 0 0 0 34 41 75 0.887 12:00 PM Volume 0 0 0 0 0 0 0 162 104 266 0.887 12:00 PM Volume 0 0 0 0 0 0 0 130 AM 11:30 AM 12:45 PM - Peak 1 of 1 11:30 AM 12:00 PM Volume 0 0 0 0 0 0 0 0 130 AM 11:30 AM 12:00 PM Volume 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				2:45 PM	- Peak 1	of 1				1				I			1	
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12:00		_			U	_			200	1	_		020		_	_	١,	, , , ,
Volume	+	0.0	0.0	0.0											• • •			
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High Int. Volume																		0.891
Volume						11:30 A	M			12:00 F	м							0.001
Peak Factor		^	n	n	n			41	75			0	168				i	
By Approach 11:00 AM		•	Ū	·	•		•			02		Ū		1				
By Approach 11:00 AM)l- 11 -	44.00	N. I. I. A.	0.45 DIA	Deel 4													
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Intersection 04:30 PM						1			*	•				•			'	
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Percent 0.0 0.0 0.0 0.0 0.0 0.0 61.3 38.7 51. 94.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0				_	_	_	444	00	000	,,	000		670	_		^		000
05:00			_	_	U	_			230				6/3				0	903
Volume Peak Factor High Int. Volume 0 0 0 0 0 0 35 35 70 7 195 0 202 Peak Factor 0.830 Peak Hour From 04:00 PM to 05:45 PM - Peak 1 of 1 By Approach 04:00 PM Volume 0 0 0 0 0 0 0 136 103 239 34 639 0 673 0 0 0 0 0 Percent 0 0.0 56.9 43.1 5.1 94.9 0.0 High Int Volume 0 35 35 70 7 195 0 202		0.0	0.0	Ų.U		0.0	61.3	36.7		5.1	94.9	0.0		0.0	0.0	0.0		
Peak Factor		0	0	0	0	0	35	35	70	7	195	0	202	0	0	0	0	272
High Int. Volume 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		0.000
Volume 0 0 0 35 35 70 7 195 0 202 202 202 203						05.00 5				05,00 5	20.0							0.630
Peak Factor 0.821 0.833 Peak Hour From 04:00 PM to 05:45 PM - Peak 1 of 1 By Approach 04:00 PM 04:15 PM 04:30 PM 04:00 PM Volume 0 0 0 0 0 0 Percent High Int Volume 0 35 35 70 05:00 PM 7 195 0 202		0	^	^	0			25	70			0	202					
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By Approach 04:00 PM	Peak Factor					l			0.821	I			0.833				1	
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High Int 05:00 PM 05:00 PM 0 35 35 70 7 195 0 202		0	0	0	0				239				673	0	0	0	0	
Volume 0 35 35 70 7 195 0 202		-	-	-				43.1				0.0		-	-	-		
		-										_		-				
Peak Factor - 0.854 0.833		-	-	-		0	35	35		7	195	0		-	-	-		
	Peak Factor				-				0.854				0.833				-	

File Name: Franklin St. & Nash S

Site Code : 01321106 Start Date : 5/27/2003

Page No : 1

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							Groups	s Printed-	Unshift	ed				kl	sh St.		
			klin St.			Man	tbound	1		North	bound				sn St. :bound		
		South	nbound	App.				Арр.				App.				Арр.	Int.
Start Time	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	11	29	0	40	0	0	0	0	0	0	0	0	0	10	1	11	51
07:15 AM	18	42	0	60	0	0	0	0	0	0	0	0	0	13	2	15	75
07:30 AM	26	40	0	66	0	0	0	0	0	0	0	0	0	28	3	31	97
07:45 AM	37	67	0_	104	0	0	0	0	0	0	0	0	0	22	. 5	27	131
Total	92	178	0	270	0	0	0	0	0	0	0	0	0	73	11	84	354
08:00 AM	45	45	0	90	0	0	0	0	0	0	0	0	0	18	3	21	111
08:15 AM	26	76	0	102	0	0	0	0	0	0	0	0	0	16	9	25	127
08:30 AM	30	57	0	87	0	0	0	0	0	0	0	0	0	10	3	13	100
08:45 AM	27	58	0	85	0	0	0	0	0	0	0	0	0	20	4	24	109
Total	128	236	0	364	0	0	0	0	0	0	0	Ö	0	64	19	83	447
11:00 AM	32	46	0	78	0	0	0	0	0	0	0	0	0	19	9	28	106
11:15 AM	29	66	0	95	0	0	0	0	0	0	0	0	0	23	3	26	121
11:30 AM	34	65	0	99	0	0	0	0	0	0	0	0	0	11	3	14	113
11:45 AM	35	69	0	104	0	0	0	0	0	0	0	0	0	14	5	19	123
Total	130	246	0	376	0	0	0	0	0	0	0	0	0	67	20	87	463
12:00 PM	46	61	0	107	0	0	0	0	0	0	0	0	0	23	5	28	135
12:15 PM	35	82	0	117	0	0	0	0	0	0	0	0	0	35	3	38	1 5 5
12:30 PM	33	73	0	106	0	0	0	0	0	0	0	0	0	30	7	37	143
12:45 PM	34	82	0	116	0	0	0	0	0	0	0	0	0	45	7	52	168
Total	148	298	0	446	0	0	0	0	0	0	0	0	0	133	22	155	601
04:00 PM	30	69	0	99	0	0	0	0	0	0	0	0	0	20	4	24	123
04:15 PM	31	75	0	106	0	0	Ō	0	0	0	0	0	0	16	3	19	125
04:30 PM	31	53	0	84	0	0	0	0	0	0	0	0	0	17	3	20	104
04:45 PM	23	68	0	91	0	0	0	0	0	0	0	0	0	10	3	13	104
Total	115	265	0	380	0	0	0	Ō	0	0	0	0	0	63	13	76	456
05:00 PM	39	108	0	147	0	0	0	0	0	0	0	0	0	10	7	17	164
05:15 PM	19	73	0	92	0	0	0	0	0	0	0	0	0	11	6	17	109
05:30 PM	25	64	0	89	0	0	0	0	0	0	0	0	0	7	9	16	105
05:45 PM	14	39	0	53	0	0	0	0	0	0	0	0	0	6	5	11	64
Total	97	284	0	381	0	0	0	0	0	0	0	0	0	34	27	61	442

0

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15.7

112

20.5

4.1

546 |

19.8

2763

Grand Total

Apprch %

Total %

1507

68.0

54.5

710

32.0

25.7

0

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0.0

2217

80.2

0

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0.0

0.0

0.0

File Name: Franklin St. & Nash S

Site Code : 01321106 Start Date : 5/27/2003

			klin St. ibound			West	tbound			North	bound				sh St. bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
eak Hour Fron			3:45 AM	- Peak 1	of 1				1				ı				
Intersection			^	202		^	^	^	0	0	0	^	0	66	20	86	469
Volume	138	245	0 0.0	383	0.0	0.0	0.0	0	0.0	0.0	0.0	0	0.0	76.7	23.3	50	403
Percent 07:45	36.0	64.0	Ų.U		0.0	0.0	0.0										
Volume	37	67	0	104	0	0	0	0	0	0	0	0	0	22	5	27	131
Peak Factor																	0.895
High Int.	07:45 A	М			6:45:00	AM (6:45:00	AM (07:45 A	AM.			
Volume	37	67	0	104	0	0	0	0	0	0	0	0	0	22	5	27	
Peak Factor				0.921												0.796	
eak Hour Fron			B:45 AM	- Peak 1												1	
By Approach			_		07:00 A		_		07:00 A		_		07:30 A			404	
Volume	138	245	0	383	0	0	0	0	0	0	0	0	0	84	20	104	
Percent	36.0	64.0	0.0		-	-	-		-	-	-		0.0	80.8	19.2		
High Int.	07:45 A		^	104	-				-				07:30 A	1M 28	3	31	
Volume Peak Factor	37	67	0	104 0.921	-	-	-	-	-	-	-	-	"	20	3	0.839	
eak Hour Fror			2:45 PM	- Peak 1	of 1				1				I			l	
Volume	148	298	0	446	0	0	0	0	0	0	0	0	0	133	22	155	601
Percent	33.2	66.8	0.0		0.0	0.0	0.0	_	0.0	0.0	0.0		0.0	85.8	14.2		
12:45	04		•	440	0	^	0	0	0	0	0	0	۰ ا	45	7	52	168
Volume	34	82	0	116	0	0	U	U	٠ ا	U	U	U	"	40	,	52	100
Peak Factor																	0.894
High Int.	12:15 P								_	_	_		12:45 F		_		
Volume Peak Factor	35	82	0	117 0.953	0	0	0	0	0	0	0	0	0	45	7	52 0.745	
eak Hour Fron	n 11:00 A	AM to 12	2:45 PM	- Peak 1	of 1												
By Approach			•		11:00 A	M			11:00 A	\M			12:00 F	M		i	
Volume	148	298	0	446	0	0	0	0	0	0	0	0	0	133	22	155	
Percent	33.2	66.8	0.0		-	-	-		-	-	-		0.0	85.8	14.2		
High Int.	12:15 P				-				-				12:45 F			į	
Volume	35	82	0	117	-	-	-	-	-	-	-	-	0	45	7	52	
Peak Factor				0.953				-				-				0.745	
eak Hour Fron			5:45 PM	- Peak 1	of 1				İ				I			1	
Volume	124	304	0	428	lo	0	0	0	٥ ا	0	0	0	0	53	16	69	497
Percent	29.0	71.0	0.0	420	0.0	0.0	0.0	v	0.0	0.0	0.0	Ū	0.0	76.8	23.2		401
05:00								_				_					
Volume	39	108	0	147	0	0	0	0	0	0	0	0	0	10	7	17	164
Peak Factor																	0.758
High Int.	05:00 P	M											04:30 F	M			
Volume	39	108	0	147	0	0	0	0	0	0	0	0	0	17	3	20	
Peak Factor				0.728					İ							0.863	
eak Hour Fron	n 04:00 F	PM to 0	5:45 PM	- Peak 1													
By Approach					04:00 F	M			04:00 F	M			04:00 F			1	
Volume	124	304	0	428	0	0	0	0	0	0	0	0	0	63	13	76	
Percent	29.0	71.0	0.0		-	-	-		-	-	-		0.0	82.9	17.1		
High Int.			_		-				-				04:00 F				
Volume	39	108	0	147	-	-	-	-	-	-	-	-	0	20	4	24	
Peak Factor				0.728				-				-	l			0.792	

Site Code : 01321101 Start Date : 5/20/2003

File Name: Church St. & Nash S

Page No : 1

Groups Printed- Unshifted

										Groups	Print	ed- Ur	shifte	d									
			rurch					Nash S				_	hurch				_	Vash S					
		So	uthbo	und			W	estbou	ınd			No	orthbo	und			E	astbou	ind		É al	la els	
Start		Thr	Rig	Pe	Арр.		Thr	Rig	Pe	App.		Thr	Rig	Pe	App.		Thr	Rig	Pe	App.	Excl	Inclu.	Int.
Time	Left	u	ht	ds	Total	Left	u	ht	ds	Total	Left	u	ht	ds	Total	Left	u	ht	ds	Total	u. Total	Tota	Total
	4.6		4.0			4.0	4.0	10	4.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		TULAI	- 1	
Factor	1.0	1.0	1.0	1.0	0	1.0	1.0 0	1.0 <u> </u> 0	1.0	0	1.0	35	1.0] 3	0	38	3	1.0	0	0	13	0	51	51
07:00 AM 07:15 AM	0	0	0	0 1	0	0	0	0	0	0	0	60	4	1	64	2	22	Ö	Ö	24	2	88	90
07:13 AM 07:30 AM	0	0	0	Ö	0	0	0	0	0	o l	Ö	65	7	Ö	72	7	24	ŏ	1	31	1	103	104
07:30 AM	0	0	0	0	Ö	0	Ö	0	Ö	ő	Ö	82	10	ő	92	7	39	Õ	ò	46	ò	138	138
Total	0	0	-0	1	 6	Ö	ŏ	0	- 0	0	0	242	24	1	266	19	95	0	1	114	3	380	383
10101	•	•	•	•	•		_	_	•	- 1	-					'							
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	71	12	0	83	10	28	0	0	38	0	121	121
08:15 AM	Ō	0	0	0	0	0	0	0	0	0	0	55	9	1	64	6	25	0	1	31	2	95	97
08:30 AM	0	0	0	0	0	0	0	0	1	0	0	69	7	2	76	13	13	0	0	26	3	102	105
08:45 AM	0	0	0	1	0	0	0	0	1	0	0	90	8	1_	98	8	6	0	2	14	5	112	117
Total	0	0	0	1	0	0	0	0	2	0	0	285	36	4	321	37	72	0	3	109	10	430	440
		_	_	_			_	_	_	•	۱ ۵	77			0.4	40	47	٥	^	36 l	4	117	121
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	77 68	4	4	81 72	19 18	17 19	0	0	37	4	109	113
11:15 AM	0	0	0	0	0	0	0	0	2 1	0	0	76	8	2	84	15	17	0	0	32	1	116	117
11:30 AM	0	0	0	0	0	0	0	0	Ó	0	0	88	12	0	100	23	32	0	o o	55	ò	155	155
11:45 AM Total	<u>0</u>	- 0		0	0	0	- 0	.	3	Ö	0	309	28	6	337	75	85	0	ŏ	160	9	497	506
10(2)	U	·	·	U	·	, 0		Ū	•	O ;		000	20	•	00.	,	-	•	•	.00			555
12:00 PM	0	0	0	1	0	0	0	0	0	0	0	88	13	0	101	26	21	0	1	47	2	148	150
12:15 PM	ō	Ō	ō	Ó	Ō	0	Ō	Ō	0	0	0	79	9	1	88	24	19	0	0	43	1	131	132
12:30 PM	0	Ō	Ó	0	0	0	0	0	0	0	0	58	15	1	73	23	27	0	0	50	1	123	124
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	70	7	1	77	25	39	0	2	64	3	141	144_
Total	0	0	0	1	0	0	0	0	0	0	0	295	44	3	339	98	106	0	3	204	7	543	550
	_	_	_					_					_		0.4		0.5	_	^	ا مد	_	400	400
04:00 PM	0	0	0	1	0	0	0	0	1	0	0	88	6	1	94	17	25	0	0	42	3	136	139
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	82 90	13 9	1	95 99	17 17	27 24	0	1	44 41	2	139 140	141 140
04:30 PM	0	0	0	0	0	0	0	0	0 2	0	0	91	10	3	101	13	23	0	ő	36	5	137	142
04:45 PM Total	0	0	0	1	0	0	0	0	3	0	0	351	38	5	389	64	99	0	1	163	10	552	562
Total	U	υ	U	1	U	0	v	U	J	U	, 0	551	30	3	303	1 04	55	U	'	100	10	JUZ	00Z
05:00 PM	0	0	0	0	0	l 0	0	0	1	0	0	121	16	0	137	11	40	0	0	51	1	188	189
05:15 PM	ŏ	ŏ	ŏ	Õ	ŏ	Ö	ŏ	Õ	Ó	ŏ	ō	81	11	ō	92	11	17	ŏ	ō	28	Ó	120	120
05:30 PM	ŏ	ŏ	ŏ	1	ŏ	Ō	Ŏ	Õ	1	ŏ	ō	66	6	1	72	5	33	ō	4	38	7	110	117
05:45 PM	ō	ō	Ŏ	2	Ŏ	Ŏ	Ŏ	Ō	Ó	Ō	0	55	11	0	66	7	12	0	0	19	2	85	87
Total	ō	- 0	0	3	ō	Ö	Ō	0	2	0	0	323	44	1	367	34	102	0	4	136	10	503	513
-	•	-				•				'													
Grand	0	0	0	7	0	0	0	0	10	0	0	180	214	20	2019	327	559	0	12	886	49	2905	2954
Total	U	U	U	′	U	"	U	U	10			5		20	2013			v	12	300	70	2000	2007
Apprch %	0.0	0.0	0.0			0.0	0.0	0.0			0.0	89.	10.			36.	63.	0.0					
Apploit 70	0.0	0.0	Ų.U			5.5	0.0	0.0			0.0	4	6			9	1	0.0					
Total %	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	62.	7.4		69.5	11.	19.	0.0		30.5	1.7	98.3	
						`~						1				3	2					•	

File Name : Church St. & Nash S

Site Code : 01321101 Start Date : 5/20/2003

			rch St.				sh St. tbound				rch St.				sh St.		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
eak Hour Fron	n 07:00	AM to 0	8:45 AM	- Peak 1	of 1												
Intersection	07:30 /	AM .													_		
Volume	0	0	0	0	0	0	0	0	0	273	38	311	30	116	0	146	457
Percent	0.0	0.0	0.0		0.0	0.0	0.0		0.0	87.8	12.2		20.5	79.5	0.0	İ	
07:45	0	0	0	0	l 0	0	0	0	٥	82	10	92	7	39	0	46	138
Volume	U	U	Ų	U	"	U	Ū	U	"	02		Ų.	,		•		-
Peak Factor																	0.828
High Int.	6:45:00	MA C			6:45:00	AM			07:45 A	M			07:45 A	M			
Volume	0	0	0	0	0	0	0	0	0	82	10	92	7	39	0	46	
Peak Factor												0.845				0.793	
'eak Hour Fron			8:45 AM	- Peak 1	of 1												
By Approach	07:00 /	AΜ			07:00 A	M			08:00 A	M			07:30 A			İ	
Volume	0	0	0	0	0	0	0	0	0	285	36	321	30	116	0	146	
Percent	-	-	-		-	-	-		0.0	88.8	11.2		20.5	79.5	0.0		
High Int.	_				-				08:45 A	MA			07:45 A	M			
Volume	-	-	-	_	-	-	-	-	0	90	8	98	7	39	0	46	
Peak Factor				-				-				0.819				0.793	
eak Hour Fron	n 11:00	AM to 1	2:45 PM	- Peak 1	of 1											,	
Intersection	11:45	AΜ															
Volume	0	0	0	0	0	0	0	0	0	313	49	362	96	99	0	195	557
Percent	0.0	0.0	0.0		0.0	0.0	0.0		0.0	86.5	13.5		49.2	50.8	0.0		
11:45	0	0	0	0	0	0	0	0	0	88	12	100	23	32	0	55	155
Volume	U	0	U	U	"	U	U	U	"	00	12	100	2.5	32	U	33	100
Peak Factor																ļ	0.898
High Int.									12:00 F	PM			11:45 A	M		l	
Volume	0	0	0	0	0	0	0	0	0	88	13	101	23	32	0	55	
Peak Factor												0.896				0.886	
eak Hour Fron	n 11:00	AM to 1	2:45 PM	- Peak 1	of 1												
By Approach	11:00 /	AM			11:00 A	M			11:30 A	AM/			12:00 F	M		i	
Volume	0	0	0	0	0	0	0	0	0	331	42	373	98	106	0	204	
Percent	-	-	-		-	-	-		0.0	88.7	11.3		48.0	52.0	0.0	İ	
High Int.	-				-				12:00 F	PM			12:45 F	M			
Volume	_	-	-	-	-	-	-	-	0	88	13	101	25	39	0	64	
Peak Factor				-				-				0.923				0.797	
eak Hour Fron	n 04:00	PM to 0	5:45 PM	- Peak 1	of 1												
Intersection	04:15	PM															
Volume	0	0	0	0	0	0	0	0	0	384	48	432	58	114	0	172	604
Percent	0.0	0.0	0.0		0.0	0.0	0.0		0.0	88.9	11.1		33.7	66.3	0.0		
05:00	0	0	0	0	0	0	0	0	0	121	16	137	11	40	0	51	188
Volume	U	U	U	v	"	U	U	J	, ,	121	10	101	''	70	J	٠.١	
Peak Factor									-								0.803
High Int.									05:00 F				05:00 F	M		1	
Volume	0	0	0	0	0	0	0	0	0	121	16	137	11	40	0	51	
Peak Factor												0.788				0.843	
'eak Hour Fron	n 04:00	PM to 0	5:45 PM	- Peak 1													
By Approach	04:00	PM			04:00 F	PM			04:15 F				04:15 F]	
Volume	0	0	0	0	0	0	0	0	0	384	48	432	58	114	0	172	
Percent	_	-	-		_	-	-		0.0	88.9	11.1		33.7	66.3	0.0	1	
High Int.	_				-				05:00 F				05:00 F			1	
Volume	_	-	-	-	-	-	_	-	0	121	16	137	11	40	0	51	
volume												0.788				0.843	

APPENDIX C: LAND USE DATA AND TRIP GENERATION

Land Use and Trip Generation Detail

																											Second			Second
	No.		First Floor	First Floor	First Floor	First Floor	First Floor	First Floor	First Floor	Second	Second	Second	Second Floor AM Peak	Second	Second Floor PM	Second Floor PM		Third	Third	Third Floor AM	Third Floor PM		Third Floor PM	Floor	Second Floor	Second Floor	Floor Office AM	Second Floor	Second Floor Office PM	Floor Office PM
Building Pin No.	Floors	blockid	Daily Trips		Out	Total	In Peak	Out Peak			Peak In				Peak Out		Daily Trips		Floor AM Peak Out			Peak Out	Peak Total	Daily Trips	Peak In	Office AM Peak Out	Total		Peak Out	
375907587613 375925589950	1	110 110																												
375925598214 375925598373	1	108 108																												
375925599575	1	108																												
375925690159	2	108 108	856 373	63 28	69 30	132 58	23 10	33 14	56 24	242 182	2	10 6	12 7	19 15	9	28 23								70 31	9	1	10 5	2	8	10 4
375925690259 375925690273	2	108	373	28	25	47	8	12	20	170	1	5	6	15	7	23								25	3	0	3	1	3	4
375925690450	2	108	50	4	4	8	1	2	3	140	1	3	4	13	6	19														
375925690489 375925690674	2	108 105	46 168	12	13	7 26	1 5	6	11	152	1	4	5	14	7	20								14	2	0	2	0	2	2
375925690698	2	105	218	16	18	34	6	8	14	158	1	4	5	14	7	21								18	2	0	2	0	2	2
375925690803 375925690930	2	105 105								140 194	1 1	<u>3</u>	<u>4</u> 8	13 16	<u>6</u> 8	19 24														
375925691228	2	108	93	7	7	14	3	4	6	146	1	4	4	13	7	20								8	1	0	1	0	1	1
375925691341 375925691367	2	108 108	249	18	20	38	7	10	16	164 170	1 1	5 5	6	14 15	7	21								20	3	0	3	0	2	2
375925691471	2	108								158	1	4	5	14	7	21														
375925691495	2	108	407	0	40	20	2	-	0	158	11	4	5	14	7	21								10	1	0	4	0		
375925691701 375925691726	2	105 105	127 118	9	10 9	20 18	3	5 5	8	152 146	1	4	5 4	14 13	7	20								10	1	0	1	0	1	1
375925691729	2	105								146	1	4	4	13	7	20														
375925691831 375925691845	1	105 105	79	6	6	12	2	3	5	146	1	4	4	13	7	20														
375925691856	2	105						-	-	158	1	4	5	14	7	21														
375925691970 375925692500	2	105 108	135	10	11	21	4	5	9	152 164	1 1	- 4 - 5	5 6	14 14	7 7	20								11	1	0	1	0	1	1
375925692913	1	105	46	3	4	7	1	2	3	104		<u> </u>		19		۷1														
375925692938 375926692538	1	105 106								158		4	5	14	7	21														
375926692641	2	106	110	8	9	17	3	4	7	146	1	4	4	13	7	20								9	1	0	1	0	1	1
375926692655	2	106	213	16	17	33	6	8	14	158	1	4	5	14	7	21								17	2	0	2	0	2	2
375926692669 375926692782	2	106 106								158 152	1 1	4	<u>5</u>	14 14	7 7	21														
375926692785	2	106								152	1	4	5	14	7	20														
375926692797 375926693709	2	106 106								152 146	1 1	4	5 4	14	7	20														
375926693812	2	106								152	1	4	5	14	7	20														
375926693837	2	106								176	1	6	7	15	7	23														
375967194500 375968107500	1	220 218	10	1	1	1	0	0	1																					
375968111500	1	218	163	12	13	25	4	6	11																					
375968139800 375968210000	1 1	218 218	30 97	7	2 8	5 15	3	<u>1</u> 4	6																					
375968245400	1	113	48	4	4	7	1	2	3																					
375968259400 375968341700	1 1	113 113																												
375968499500	1	112	258	19	21	40	7	10	17																					
375969402000	2	112	37	3	3	6	1 4	1 6	2	450	1		5	4.4	7	20								42	2	0	2	0	1	
375969439000 375969512400	1	109 109	158 168	12 12	13 13	24 26	5	6	10 11	152	1	4	5	14		20								13	2	U		U	- 1	
375969515100	1	109	84	6	7	26 13	2	3	6																					
375969516700 375969528700	1 1	109 109	55 143	11	11	9 22	4	6	9																					
375969531400	2	109	207	15	17	32	6	8	14	158	1	4	5	14	7	21								17	2	0	2	0	2	2
375969532900 375969543200	2 2	109 109	148 120	11 9	12 10	23 19	3	<u>6</u> 5	10 8	152 146	1 1	4	5 4	14 13	7	20 20								12 10	2	0	1	0	1 1	1 1
375969549600	2	109	120		10	.3	<u> </u>	<u> </u>	<u> </u>	176	1	6	7	15	7	23								10		, , , , , , , , , , , , , , , , , , ,				
375969558900 375969631800	3	107 109	194	14	16	30	5	7	13	111 70	13 8	2	14 9	14 14	69 68	84 82	158 182	1	6	5	14 15	7 8	21 23							
375969656600	2	109	194	14	10	30	ð	,	13	176	1	6	7	15	7	23	102	- 1	0		15		23							
375969660700 375969669400	1 2	107 107	272 125	20 9	22 10	42 19	7	10 5	18	152		4	_	14	7	20								10		_		•		
375969672000	3	107	125 223	17	10	19 34	6	9	8 15	79	9	1	5 10	14	68	82	146	1	4	4	13	7	20	10	1	U	11	U	ı	
375969672300	3	107	207	15	17	32 14	6	8	14	74	8	1	9	14	68	82	146	1	4	4	13	7	20							
375969673500 375969674800	2	107 107	88	6	7	14	2	3	6	146 146	1 1	4	4	13 13	7	20														
375969685100	2	107								146	1	4	4	13	7	20														
375969686300 375969687800	2 2	107 107	102	8	8	16	3	4	7	146 140	1 1	3	4	13 13	7 6	20 19								8	111	0	11	0	11	1
375969687900	2	107								140	1	3	4	13	6	19														
375969698200 375969760600	2	107 107	159	12	13	24	4 2	6	10	152 140	1	4	5 4	14 13	7 6	20 19								13	2	0	2	0	1	1
375969760600	2	107	73	5	6	11	2	3	5	140	1	4	4	13	7	20														
375969891600	3	107	149	11	12	23	4	6	10	58	6	1	7	14	67	81	170	1	5	6	15	7	22							
375979494600 385019600073	1 1	112 105																												
385019601041	1	105	274	20	22	42	7	11	18																					
385019602679 385019602727	1	103 103	176	13	14	27	5	7	12	158	1	4	5	14	7	21								14	2	0	2	0	2	2
303019002727	1	103																												

Land Use and Trip Generation Detail

Building Pin No.	No. Floors		First Floor Daily Trips	AM Peak In	AM Peak Out	AM Peak Total	PM Peak In	First Floor PM Peak Out	PM Peak Total	Floor Daily Trips	Peak In	Floor AM Peak Out	Total	Floor PM Peak In	Floor PM Peak Out		Floor	Third I Floor AM Peak Out	Floor PM	Third Floor PM Peak Out	Third Floor PM Peak Total	Daily Trips		Second Floor Office AM Peak Out	Total	Office PM	Second Floor Office PM Peak Out	Total
385019603501	2	103	779	58	63	120	21	30	51	230	2	9	11	18	9	27						64	8	1	9	1	7	8
385060704800	1	104								450						04												
385060711000 385060714400	1	104 107	0.5	_	_		1	1	2	158	11	4	5	14	7	21												
385060714400	1	107	35 129	10	3 10	5 20	3	5	8																			
385060714700	1	104	66	5	5	10	2	3	4																			
385060718000	1	104	113	8	9	17	3	4	7																			
385060718000	2	104	322	24	26	50	9	12	21	176	1	6	7	15	7	23						26	3	0	3	1	3	4
385060800900	1	104	93	7	7	14	2	4	6	170		0		13		23						20	3	- 0	3		3	
385060803800	1	104	33			14			0																			
385060805800	2	104	93	7	7	14	2	4	6	146	1	4	4	13	7	20						8	1	0	1	0	1	1
385060813500	1	104	80	6	6	12	2	3	5																			
385060815300	2	104	11	1	1	2	0	0	1	134	1	3	3	13	6	19												
385060820300	2	104	150	11	12	23	4	6	10	152	1	4	5	14	7	20						12	2	0	2	0	1	1
385060821800	2	104								140	1	3	4	13	6	19												
385060866200	1	104																										
385060918100	1	117	97	7	8	15	3	4	6																			
385070015800	1	117	267	20	21	41	7	10	17																			
385070224900	2	118								200	1	7	9	17	8	25												
385070237900	1	118																										
385070249400	1	118																										
385070258200	1	118	74	5	6	11	2	3	5																			
385070325500	1	118	601	45	48	93	16	23	39																			
385070351100	1	118	99	7	8	15	3	4	6																			
385070441100	1	118																										
385070444700	1	118																										
385082603095	2	103								152	11	4	5	14	7	20												
385082603130	1	103																										
385082603160	2	103	178	13	14	27	5	7	12	158		4	5	14	7	21						15	2	0	2	0	2	2
385082604101	2	103								152	1	4	5	14	7	20												
385082604113		103	47	3	4 4	7 8	1 1	2	3																			
385082604115 385082604117	1 1	103 103	50	4	4	8	1	2	3																			
385082604117	1	103																										
385082604118	2	103								146	1	4	4	13	7	20												
385082604221	2	103	83	6	7	13	2	3	5	146	1	4	4	13	7	20												
385082604245	2	103	03	0	- /	13		3	5	146	1	4	4	13	7	20												
385082604350	1	103	289	21	23	45	8	11	19	140		*	*	13	- '	20												
385082605882	1	101	203	- 41	23	40			13																			
385082606735	1	101																										
385082606962	1	101	114	8	9	18	3	4	7																			
385082616040	1	101	147	11	12	23	4	6	10																			
387620806017	1	112	42	3	3	6	1	2	3																			
114_115_116(sum)	ó	114_115_116_sum		6	29	34	35	17	53																			

APPENDIX D:

SYNCHRO ANALYSIS OUTPUT

ALL SYNCHRO ANALYSIS FILES ARE INCLUDED IN ELECTRONIC FORMAT ON THE CD-ROM THAT ACCOMPANIES THIS REPORT DOCUMENT.

APPENDIX E:

PUBLIC MEETING NOTES



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ROCKY MOUNT DOWNTOWN CIRCULATION STUDY

November 4, 2004 **Downtown Stakeholders Meeting**

Attendees

Listing of stakeholders in attendance is attached.

Ann Wall City of Rocky Mount Jonathan Boone City of Rocky Mount City of Rocky Mount Jerry Pierce

Jody Lewis Martin/Alexiou/Bryson, PLLC **Brian Wert** Martin/Alexiou/Bryson, PLLC

Jonathan Boone opened the meeting with an introduction and a presentation on the background and status of the traffic circulation study. Jody Lewis then gave a presentation on the work Martin/Alexiou/Bryson (M/A/B) had completed on the Downtown Circulation Study. Mr. Lewis' presentation focused on scenario development.

In response to the possible cross sections shown in the presentation by Mr. Lewis it was asked if all travel lanes would be kept as travel lanes. Mr. Lewis stated that the cross sections shown were only intended to show possible intersection treatments. Mid-block locations would retain on-street parking where possible.

Stakeholders also commented in response to the cross sections that there was no need to provide on-street parking on Franklin Street. Suburban type redevelopment had occurred along this corridor and plenty of parking was available in off-street locations to service businesses. Mr. Lewis pledged to examine this further to better gauge the need of on-street parking on Franklin Street.

Some stakeholders questioned to the scope of the Downtown Circulation Study. Many attendees were curious why the study limits encompassed all of downtown while the focus of redevelopment was a four-block portion of Main Street. Mr. Boone stated that this was to allow the council to examine all effects of making changes to the street network. Changes to the street network will not merely have affects at a given point but affect entire corridors and traffic patterns.

Some stakeholders questioned why the scenarios being presented did not consider Sunset Avenue and Thomas Street for conversion to two-way traffic. Mr. Lewis stated that in his professional opinion he believed that Thomas and Sunset would carry too much traffic in 2025 to be converted to two-way streets within the current street cross-section. Mr. Lewis stated that it was within the project scope that those streets could still be included in the scenarios and that

Downtown Constituents Meeting November 4, 2004

feedback from the meeting would be taken into consideration in the development of the final analysis scenarios.

Some stakeholders were skeptical of traffic volume projections, questioning the projected increase in through traffic in the downtown. Additionally they suggested that converting Thomas and Sunset to two-way facilities would push through traffic to Grand Avenue and could benefit the redevelopment of the Douglas Block. This would help in changing the character of downtown streets from thoroughfares to local streets.

There was strong desire from the downtown stakeholders to see the study expanded to include examining the effects of closing the Goldleaf Street and Bassett Street rail crossings. Mr. Boone stated that the consultants were directed by city council to examine only the rail crossings stated in the scope. Mr. Boone also stated that the traffic volumes and patterns on Goldleaf were such that the "No Build" analysis that the consultants would conduct would capture the effects of closing the Goldleaf Street crossing. City Council would take this in to account when making any decision based on the results of the M/A/B study.

In comparing the closure of the Western Avenue/Hill Street rail crossing to the closure of the Nash Street /Marigold Street rail crossing, some stakeholders preferred closing the Western Avenue/Hill Street rail crossing. The feeling was that if the Nash Street /Marigold Street rail crossing were closed, then the rail operators would allow trains to stop as far back as the Western Avenue/Hill Street rail crossing. This would prevent pedestrian traffic from crossing at the Nash Street /Marigold Street crossing.

Some of the stakeholders expressed a desire to retain the current one-way operation on Main Street. One concern raised was that converting portions of Main Street to two-way operation might require removing angled parking. Many of the downtown stakeholders expressed an aversion to converting angled parking to parallel parking. Mr. Lewis commented that the impact on existing parking is one of the factors under consideration when assessing the feasibility of converting streets to two-way operation.



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ROCKY MOUNT DOWNTOWN CIRCULATION STUDY

November 9, 2004 **Planning Board Work Session**

Attendees

Members of the Planning Board were present.

Jonathan Boone City of Rocky Mount Jerry Pierce City of Rocky Mount Bob League City of Rocky Mount Ann Wall City of Rocky Mount City of Rocky Mount Charles Penny Peter Varney City of Rocky Mount

Martin/Alexiou/Bryson, PLLC (M/A/B) Jody Lewis **Brian Wert** Martin/Alexiou/Bryson, PLLC (M/A/B)

Jonathan Boone opened the meeting with an introduction and a presentation on the background and status of the traffic circulation study. Jody Lewis then gave a presentation on the work Martin/Alexiou/Bryson (M/A/B) had completed on the Downtown Circulation Study. Mr. Lewis' presentation focused on scenario development.

In response to the possible cross sections shown in the presentation by Mr. Lewis it was asked why, if parking could be possible on Franklin Street in the future, was there not on-street parking on Franklin Street today. Mr. Lewis stated that he was not privy to the discussions that led to that decision but offered that perhaps at some point capacity consideration required three travel lanes for traffic. Jerry Pierce pointed out that in the meeting with the Downtown Stakeholders on November 4th it was stated that development on Franklin Street was more suburban in nature with sufficient off-street parking. Mr. Lewis stated that the intent of the possible crosssections being shown was to indicate areas where the City could increase the inventory of onstreet parking should the demand ever be realized.

Some members of the Planning Board questioned what the benefits were of closing a rail crossing. Mr. Boone stated that beyond safety there was little immediate benefit to closing one rail crossing. The benefits to closing multiple crossings, as is the case in Rocky Mount, is that system wide safety can be greatly increased as more attention and funding can be given to the rail crossings that do remain open. Completion of the Traffic Separation Study also guarantees that the City will receive a higher priority in terms of disbursement of state and federal funding for rail crossing safety improvement and mitigation projects.

It was asked by a member of the Planning Board how many crossings the City of Rocky Mount had closed and how many more the City had agreed to close. Mr. Boone stated that the City

Planning Board Work Session November 9, 2004

has agreed to eight crossing closures and that one more crossing was yet to be identified. M/A/B's role in the Downtown Circulation Study was to evaluate the feasibility of closing either the Nash/Marigold or the Hill/Western crossing.

A member of the Planning Board asked if there were studies available that predicted a probable economic benefit to a downtown region if one-way streets were converted to two-way streets. Mr. Boone stated it was likely that studies could be found that indicated an economic benefit and some that predicted no benefit. Mr. Boone stated that Jim Prost of Basile Baumann Prost and Associates was under contract to provide economic predictions specific to Rocky Mount.

Another member of the Planning Board asked if there were any studies examining burying the railroad tracks. Mr. Boone stated there was a study that examined elevating and/or re-routing the railroad tracks. Both scenarios are considered to be financially prohibitive.

A member of the Planning Committee stated that he had heard from visitors that the current downtown street network was difficult to navigate given the predominance of one-way streets. Additionally he stated that the network in its current form was designed to move people across downtown and not to bring people to downtown.

The following suggestions were then offered as possible revisions to the scenarios presented by Mr. Lewis:

- Examine converting Thomas Street from Raleigh Street to the rail tracks to two-way operations
- Examine converting Tarboro Street from Atlantic to George to two-way operations
- Examine converting Main Street south of Nash Street to two-way operations
- Examine closing the Goldleaf crossing and converting Thomas Street to two-way if given direction from the City Council
- Converting only one street to two-way operations is believed to have little to no benefit to the Main Street corridor
- Converting Thomas Street to two-way operations would allow traffic from the north to travel follow Peachtree Street to Franklin Street to Thomas Street to get to the Main Street corridor. This is viewed as more direct to the Main Street corridor than the current scenario that requires a more circuitous route because of the one-way operation of streets in the downtown area.
- Closing the Nash Street/Marigold Street rail crossing will prevent vehicles from using NW/SW Main Street, Nash Street/Marigold Street, NE/SE Main Street, and Thomas Street as a "loop" around downtown
- Converting Franklin Street and Church Street to two-way operations will have little impact on the economic viability of the Main Street corridor

Mr. Lewis pledged to examine these suggestions as scenario development continued.

It was asked if Basset Street had been considered for closing. Mr. Boone responded that emergency responders had intimated to the City that the it was necessary for the Basset Street crossing to remain open to ensure prompt response times to emergency situations.



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ROCKY MOUNT DOWNTOWN CIRCULATION STUDY

November 11, 2004 Stakeholders Meeting

Attendees

Listing of stakeholders in attendance is attached.

Jonathan Boone City of Rocky Mount Bob League City of Rocky Mount City of Rocky Mount Peter Varney

Jody Lewis Martin/Alexiou/Bryson, PLLC (M/A/B) **Brian Wert** Martin/Alexiou/Bryson, PLLC (M/A/B)

Jonathan Boone opened the meeting with an introduction and a presentation on the background and status of the Downtown Circulation Study. Jody Lewis then gave a presentation on the work Martin/Alexiou/Bryson (M/A/B) had completed on the Downtown Circulation Study. Mr. Lewis' presentation focused on scenario development.

A stakeholder offered the following comments:

- Closing either the Nash Street/Marigold Street or Hill Street/Western Avenue rail crossing is likely to reduce the number of people who come to downtown.
- Converting one-way streets to two-way operation will increase the vehicle queue length when vehicles are stopped awaiting trains to pass through downtown. The high train traffic will increase the chance that queue lengths will extend to Atlantic Avenue and that stopped motorists will use Atlantic Avenue to bypass the downtown all together.

Many of the stakeholders present questioned why any of the rail crossings downtown should be closed. Stakeholders then suggested that if a crossing were to be closed it would be more prudent in their opinion to close either the Goldleaf Street crossing or the Basset Street crossing. Stakeholders asked why the Nash Street/Marigold Street and the Western Avenue/Hill Street crossings were targeted. Jonathan Boone responded that the scope for the project limited the crossings that could be examined. Mr. Boone further intimated that the scope could possibly change based on public comments being gathered.

Many stakeholders present expressed a concern that closing either the Nash Street/ Marigold Street rail crossing or the Western Avenue/Hill Street rail crossing will have a negative affect on traffic on Main Street. This could be particularly acute to properties on the Edgecombe side of Main Street, severely decreasing the attractiveness of property on that side of Main Street.

A stakeholder questioned what had precipitated the Downtown Circulation Study, and what council was beholden to do upon its completion. Mr. Boone responded that the Downtown

Circulation Study was developed to study issues that had been raised as part of the Traffic Separation Study as well as other issues that had been discussed for years prior such as converting some of the downtown one-way streets to two-way operation. Mr. Boone further stated that City Council had the option to do nothing.

Another stakeholder inquired if rail crossings were closed would the city be able to leverage any additional funds. Mr. Boone responded that City Council would likely enter into resolutions with the NCDOT for additional mitigation projects and/or funding once a candidate for closure was identified. The stakeholder responded that the additional funding would not offset the effects of closing a rail crossing.

The stakeholders then inquired as to the number of crossings closed. Mr. Boone stated that 8 crossings had been identified for closure within the city limits and that the Downtown Circulation Study was to examine the feasibility and impact of closing one of the rail crossings in the downtown area.

Stakeholders were curious as to the economic impacts a rail crossing may have on the Main Street corridor. Mr. Boone stated that Jim Prost of Basile Baumann Prost and Associates is to examine the economic impacts of the scenarios M/A/B would develop. The members of the audience stated they would like to have more contact with Mr. Prost. Jody Lewis offered to provide Jim Prost with the contact information of all stakeholders present.

Stakeholders were particularly concerned that closing rail crossings would end the "quick trips" between stores. Many businesses in the downtown attract patrons when they stop to go to other stores. The audience expressed a fear that closing rail crossings would make those trips prohibitively long.

Stakeholders asked about scenarios with no closings and scenarios with only one-way to two-way conversions. Mr. Lewis stated that the "No Build" scenario would have no crossing closures and that if warranted one scenario could examine converting one-way streets to two-way operation and not closing any rail crossings.

Some stakeholders suggested that Main Street could be converted to two-way operation on both sides of the railroad tracks like it was in the past. Mr. Boone stated this would have a negative affect on traffic operations and traffic signal preemption for trains. Mr. Lewis also stated that two-way conversion on both sides of Main Street would likely require the removal of all angled parking leaving parallel parking as the only on-street parking option. Parallel parking was considered by most stakeholders present to be detrimental to business.

The stakeholders expressed strong support for the conversion of Church Street and Franklin Street to two-way operation. The audience stated that given the two-way sections on either side of downtown converting these facilities to two-way operation in the downtown area would be logical and would make traffic movements easier. Additionally, many members stated that Church Street and Franklin Street were excellent entrances into the downtown and converting these facilities to two-way operation would be very helpful to revitalizing Main Street.

One stakeholder stated repeatedly that "the cheapest thing to do was to do nothing." This stakeholder further suggested that any additional funds that were acquired be spent on projects other than changing the road network in downtown. In his opinion, the street network in downtown worked fine.



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ROCKY MOUNT DOWNTOWN CIRCULATION STUDY

February 8, 2005 Planning Board Work Session

Attendees

Josh Munden City of Rocky Mount (Planning Board Chairman)
Joseph Ray, Sr. City of Rocky Mount (Planning Board Vice-Chairman)

Keith Ballentine
Steve Evans
City of Rocky Mount (Planning Board)
City of Rocky Mount (Planning Board)
City of Rocky Mount (Planning Board)
City of Rocky Mount (Planning Board)
Edgecombe County (Planning Board)
Winslow Goins
Charles Lewis
City of Rocky Mount (Planning Board)
Edgecombe County (Planning Board)
Edgecombe County (Planning Board)

Bobbie Clark
Wayne Hill
Ken Graves
Denise Boswell
Adam Cook
Ann Wall
City of Rocky Mount (Planning Staff)
City of Rocky Mount (Planning Staff)
City of Rocky Mount (Planning Staff)
City of Rocky Mount (Planning Staff)
City of Rocky Mount (Planning Staff)
City of Rocky Mount (Planning Staff)

Jonathan Boone City of Rocky Mount Jerry Pierce City of Rocky Mount

Jody Lewis Martin/Alexiou/Bryson, PLLC (M/A/B)
Brian Wert Martin/Alexiou/Bryson, PLLC (M/A/B)

The purpose of the meeting was to present the preliminary analysis results to the planning board members during their customary work session in order to gather initial reaction and feedback on the results.

Jonathan Boone opened the meeting with general comments on the status of the study. Jonathan indicated that three meetings had been convened and that public comments have been received. These comments lead to the development of the five scenarios analyzed as part of the circulation study. Jonathan then turned to Jody Lewis to present the preliminary findings and the anticipated impacts that have thus far been determined.

Jody presented the preliminary findings. The analyses indicated that the existing signalized intersections in the study area are all operating at acceptable level of service (LOS). Traffic crossing the railroad tracks is expected to increase significantly. The analyses indicated that the combination of the increased peak hour traffic and the conversion of one-way streets to two-way operation significantly impacted traffic operations at the rail crossings.

Planning Board Work Session February 8, 2005

When asked about a final recommendation, Jody and Jonathan responded that emergency vehicle travel times, economic impacts and costing of the various alternatives were yet to be analyzed. When those aspects had been analyzed in conjunction with the preliminary traffic findings, the city council will make a final recommendation.

Members of the planning board were curious about the level of traffic analysis that had been completed. Specifically, they inquired if the additional accessibility gained by changing streets from one-way to two-way operation would make up for the lost mobility caused by new, less efficient signal phasing and the closing of one rail crossing. Jody stated that there had been no analysis completed based on travel times. Additionally, Jody stated that as part of the study no trip diversion around downtown as a result of increased congestion was assumed. This meant that the worst-case situation was analyzed for each scenario. Jody stated that a number of pass-through trips might divert to alternate routes and avoid downtown completely in order to avoid congestion in the downtown area. This could result in an improved level of service at which intersections would operate over those predicted in the analysis. To answer the planning board's question of travel time Jody pledged to examine the results of the Synchro runs further to help estimate travel times in each scenario for a common set of origins and destinations.

Members of the planning board then inquired as to the economic impact of the various scenarios. Jody stated that during project survey interviews the downtown merchants have indicated that most of their customer trips are purposeful trips; meaning the shop patrons are coming specifically to shop in the downtown shops. While congestion effects these trips less than other trips, the precise economic impact of the geographic correlation of the worst performing intersections (the rail crossings) and the major hub of downtown economic activity was not known as of yet. Jim Prost would address this situation specifically in his economic analysis. One member of the planning board offered that given the number of trains that travel through downtown on a daily basis (approximately 40), congestion and delay at the rail crossings are expected by motorists in the area and would not affect commuting patterns as significantly as increased delay at other more typical intersections.

In closing some members of the planning board reiterated their reservations about the study, as it did not include the Basset Street and Goldleaf Street rail crossings. It was the opinion of certain board members that closing either the Basset Street or Goldleaf Street rail crossings would have fewer negative impacts than closing any of the four rail crossings that are currently part of the study. Jonathan reminded the members that an option to expand the study scope to include those rail crossings in the analyses was denied by the City Council.

APPENDIX F:

IMPLEMENTATION